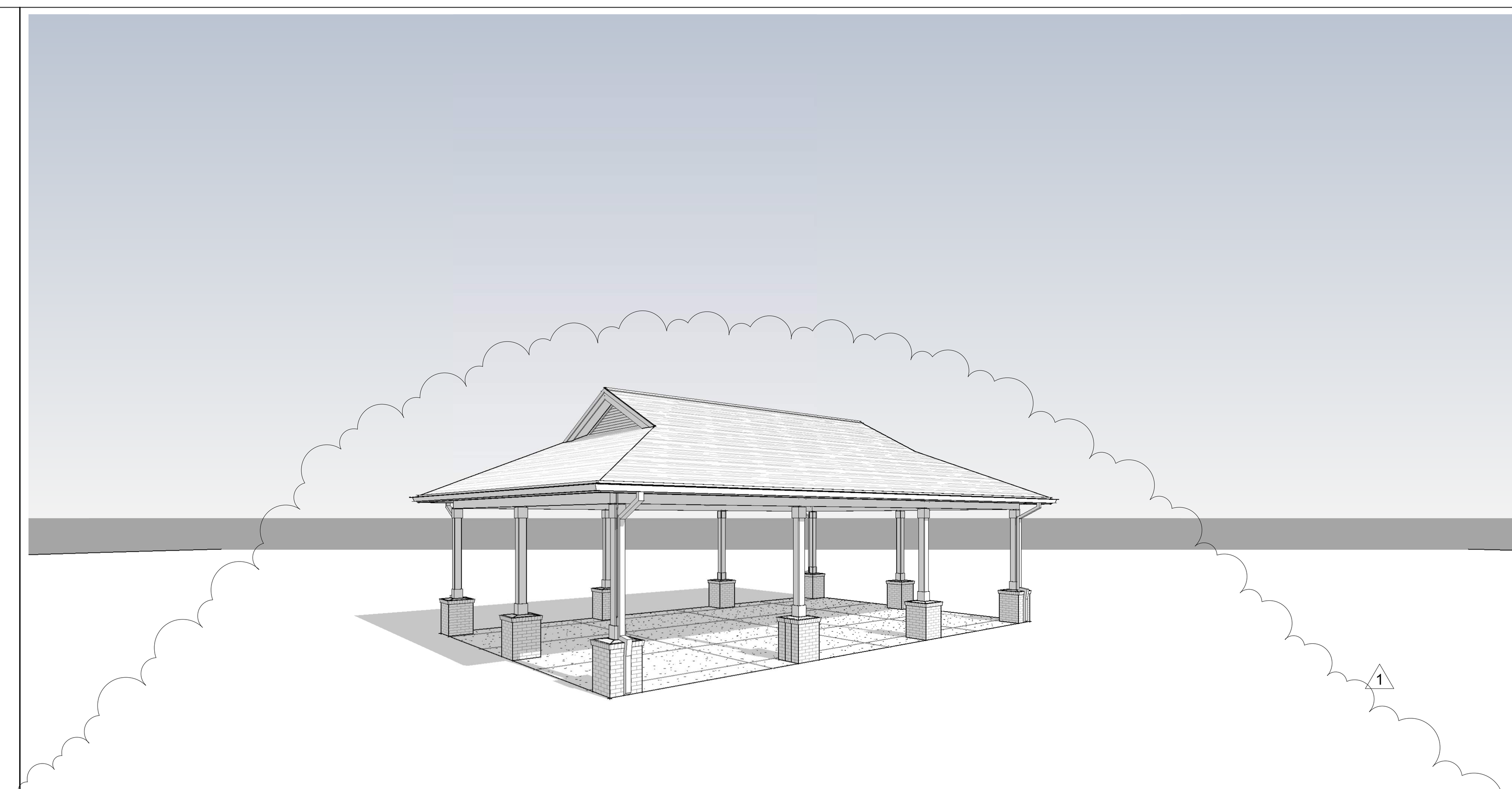
SAVANNAH QUARTERS - PAVILION

A1 PROJECT LOCATION
1" = 20'-0"



DRAWING INDEX

SHEET NUMBER	SHEET NAME	SHEET ISSUE DATE
GENERAL		
G000	COVER SHEET	06/9/2026
G100	3D VIEWS	06/9/2026
STRUCTURAL		
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S004	SPECIAL INSPECTION FORMS	06/9/2026
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S101	PLAN VIEWS	06/9/2026
S301	FOUNDATION SECTIONS & TYPICAL DETAILS	06/9/2026
S401	FRAMING SECTIONS & TYPICAL DETAILS	06/9/2026
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EC100	EXISTING CONDITIONS SITE PLAN	07/16/26
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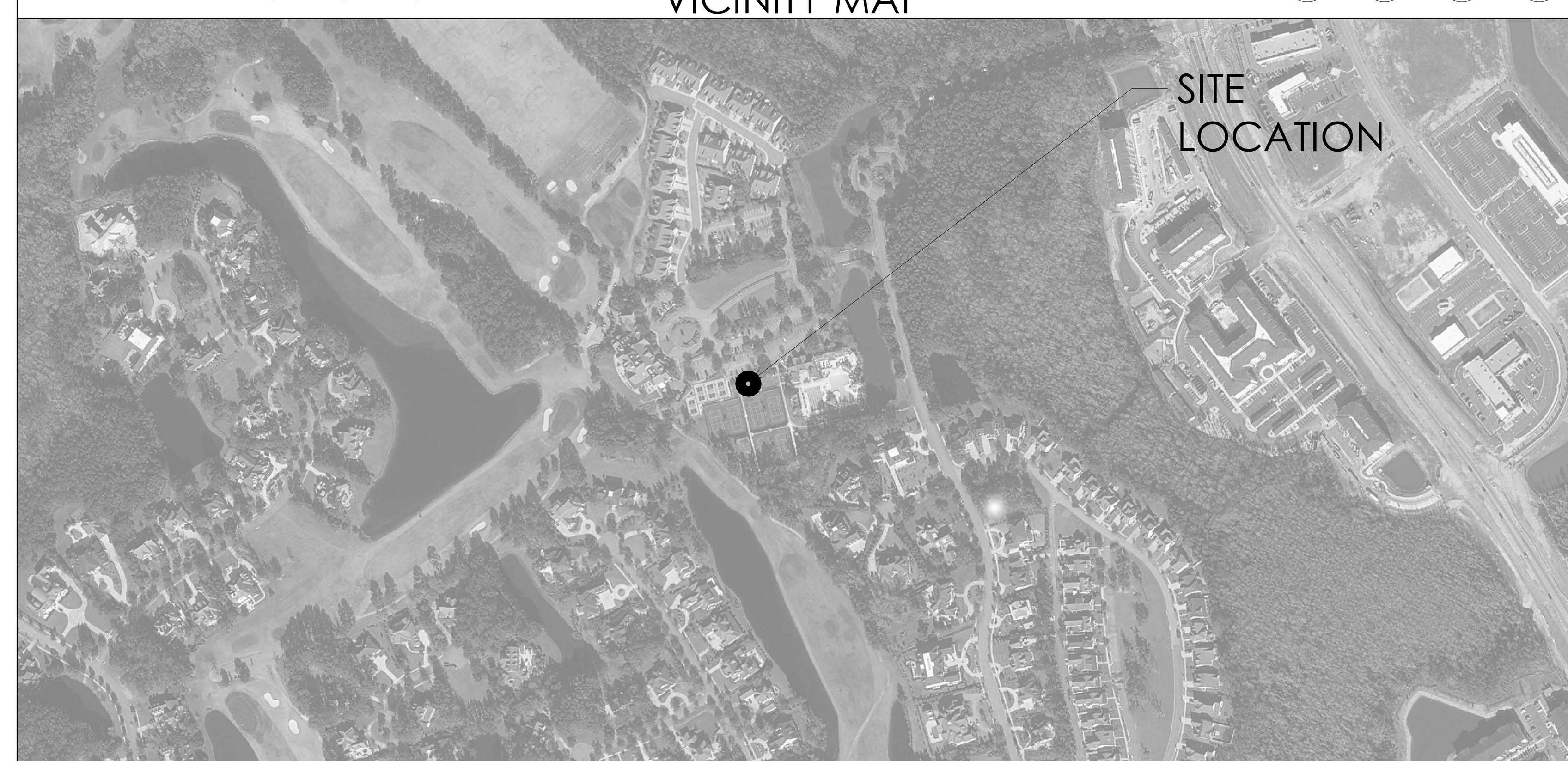
A200	BUILDING ELEVATIONS & SECTION	06/9/2026
A401	ELECTRICAL & RCP PLANS	06/9/2026

VICINITY MAP



ABBREVIATIONS

APC	ACOUSTICAL PANEL CEILING	MAX	MAXIMUM
AFF	ABOVE FINISH FLOOR	MFR	MANUFACTURER
A/C	AIR CONDITIONING	MIN	MINIMUM
AHU	AIR HANDLING UNIT	MISC	MISCELLANEOUS
CJ	CONTROL JOINT	MO	MASONRY OPENING
CL	CENTERLINE	N/A	NOT APPLICABLE
CLG	CEILING	NIC	NOT IN CONTRACT
CMU	CONCRETE MASONRY UNIT	NTS	NOT TO SCALE
CONT	CONTINUOUS	OC	ON CENTER
CT	CERAMIC TILE	OPH	OPPOSITE HAND
DIA	DIAMETER	OSB	ORIENTED STRAND BD.
DIM	DIMENSION	PERF	PERFORATED
DN	DOWN	PT	PRESSURE TREATED
DS	DOWNSPOUT	REQ'D	REQUIRED
DW	DISHWASHER	SF	SQUARE FEET
EA	EACH	SIM	SIMILAR
EQ	EQUAL	SPEC	SPECIFICATION
EQUIP	EQUIPMENT	STD	STANDARD
EXT	EXTERIOR	TBD	TO BE DETERMINED
FD	FLOOR DRAIN	T&G	TONGUE & GROOVE
FF	FINISH FLOOR	TEMP	TEMPORARY
FFE	FINISH FLOOR ELEV.	TV	TELEVISION
FIN	FINISH	TYP	TYPICAL
FT	FEET	U/C	UNDER COUNTER
GA	GAUGE	W/D	WASHER/DRYER
GWB	GYPSUM WALLBOARD	WD	WOOD
HB	HOSE BIBB	WH	WATER HEATER
INT	INTERIOR		

CONTACT

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SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE
1	Response to Permitting Comments	7/10/2026

STATE OF GEORGIA
STEVEN G. STOWERS
6/17/26
CERTIFICATE NO. 011631
REGISTERED ARCHITECT

FOR CONSTRUCTION

GENERAL CONTRACTOR
24-HOUR CONTACT

Paul Balducci - Talahi Builders
912.239.0338
PAUL@TALAHIBUILDERS.COM

General:

1. Property Address:
   8 Palladian Way
   Pooler, GA 31322

2. Gross Square Footage: 1,350 (Total Covered)

DEPARTMENT OF PLANNING & DEVELOPMENT
APPROVED BY: rjarles
DATE: 02:37 pm, Aug 20 2026

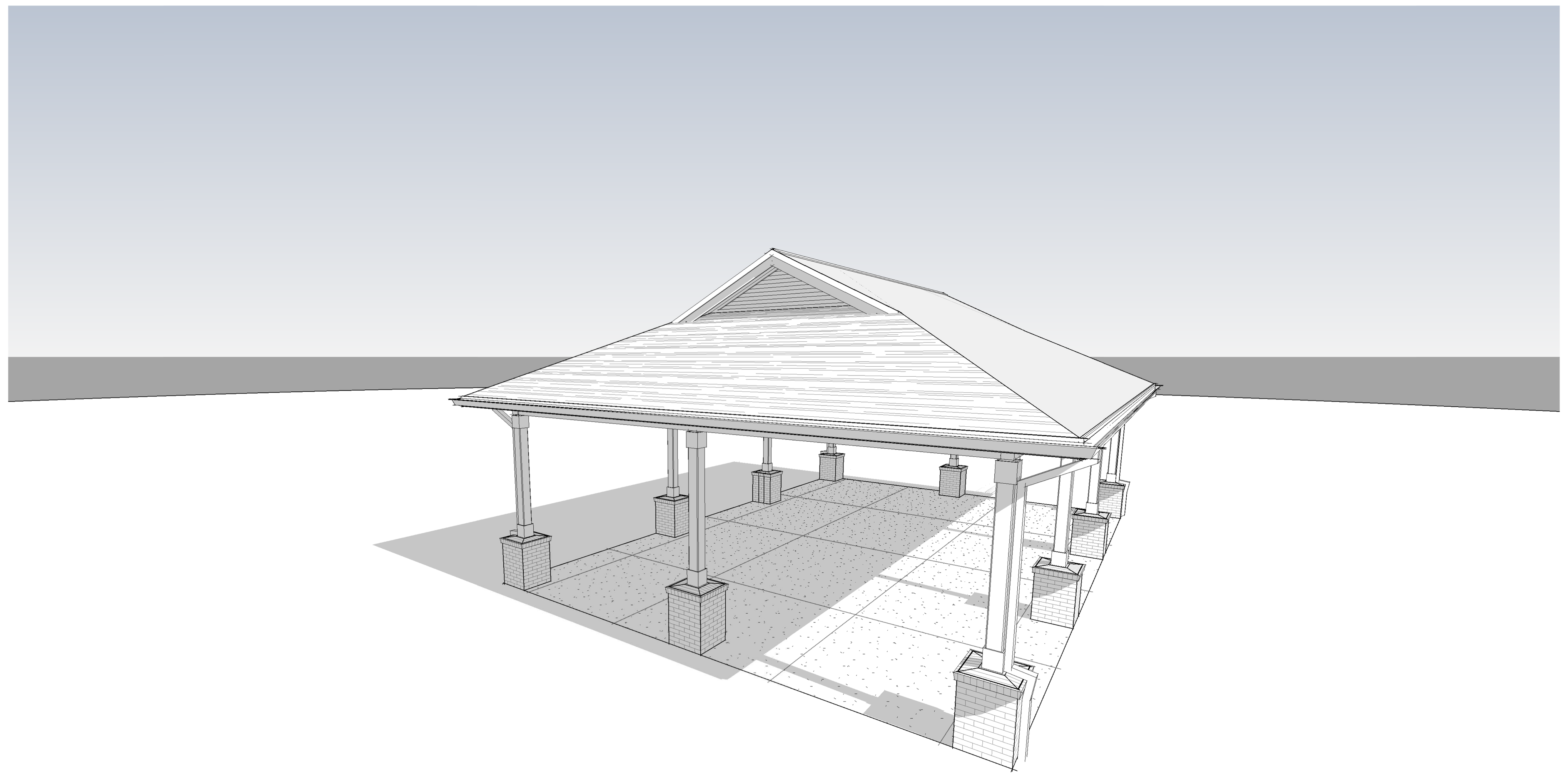
ARCHITECTURE A101
BEAUFORT - SAVANNAH
518 Martin Luther King, Jr. Boulevard | Savannah, Georgia
www.A101.Design | 912.306.0945

COVER SHEET

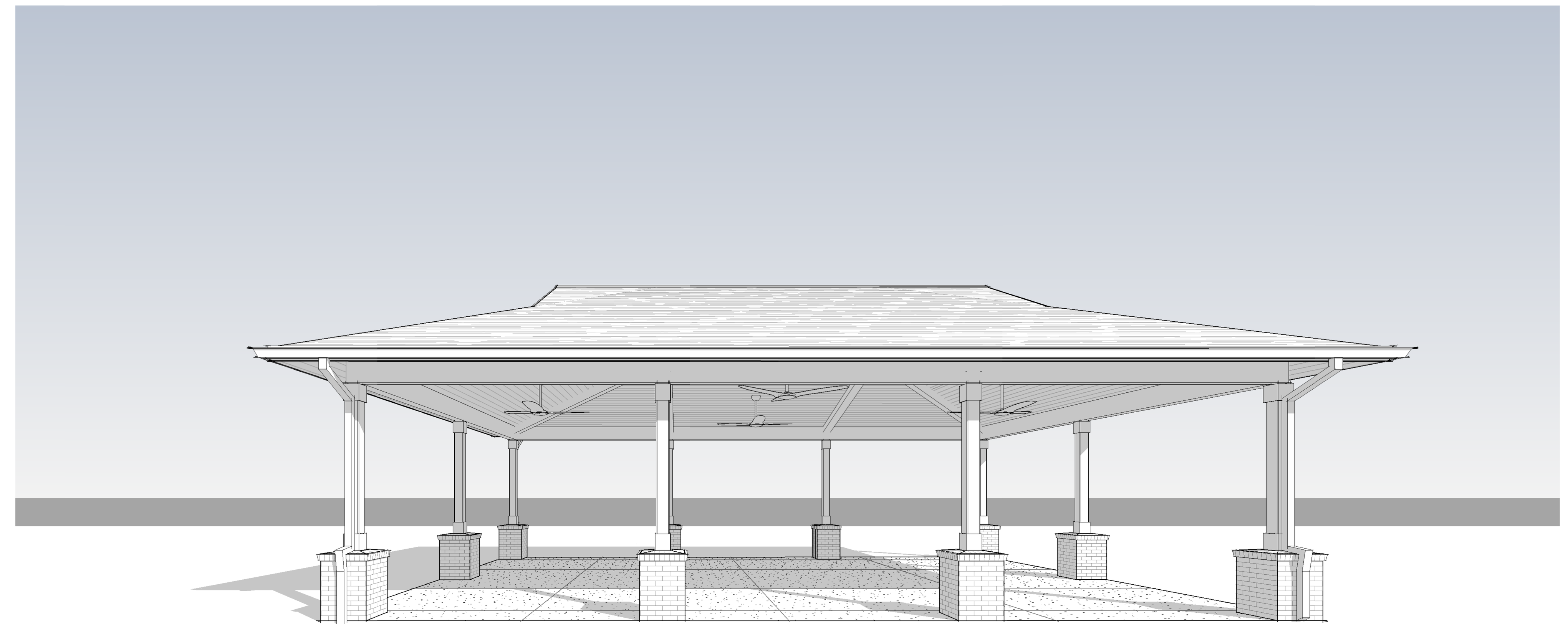
	2026.04
	06/17/2026
Drawn By	CK
Checked By	SGS

G000

7/16/2026 4:31:52 PM

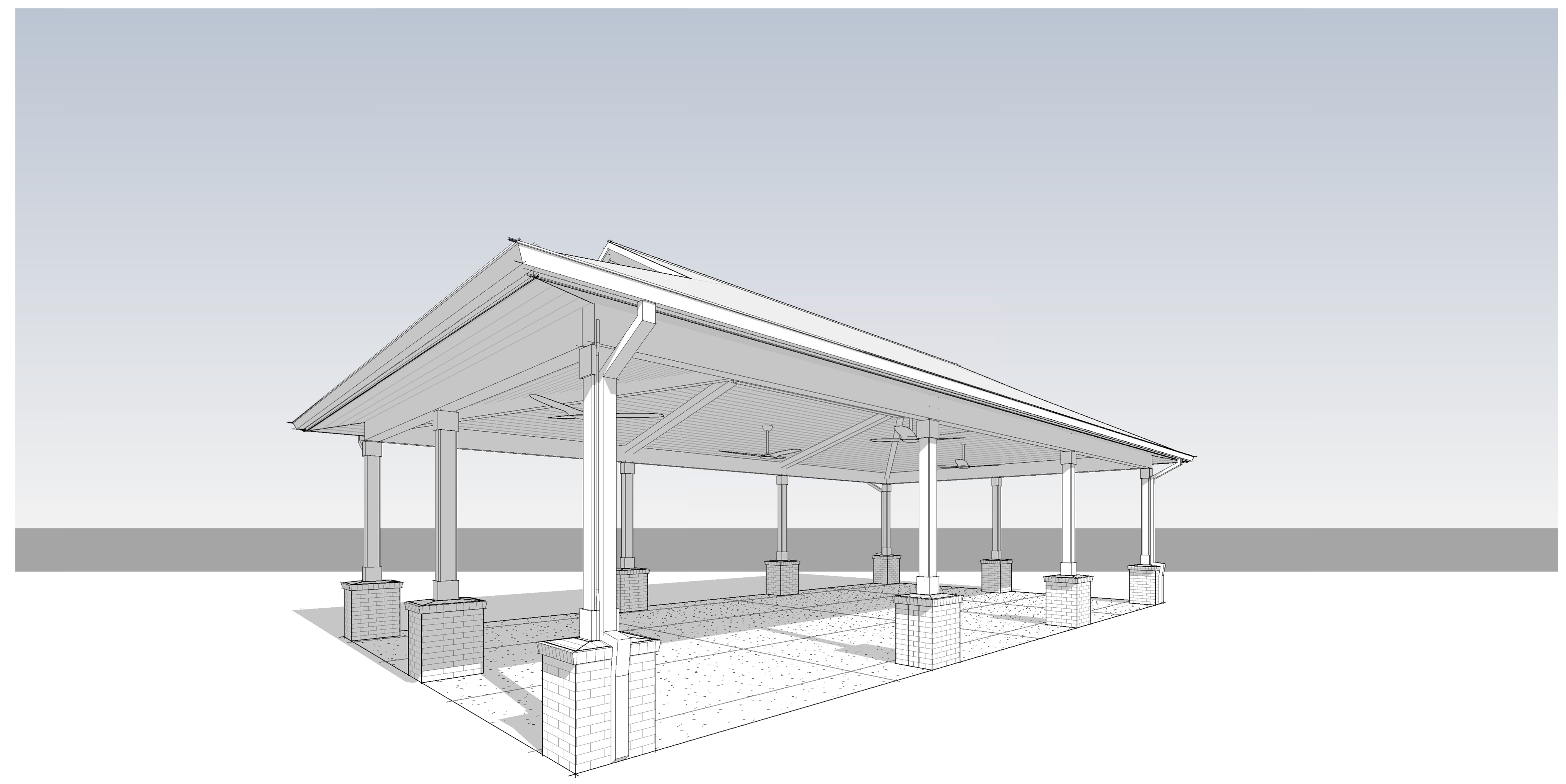


3D VIEW 01

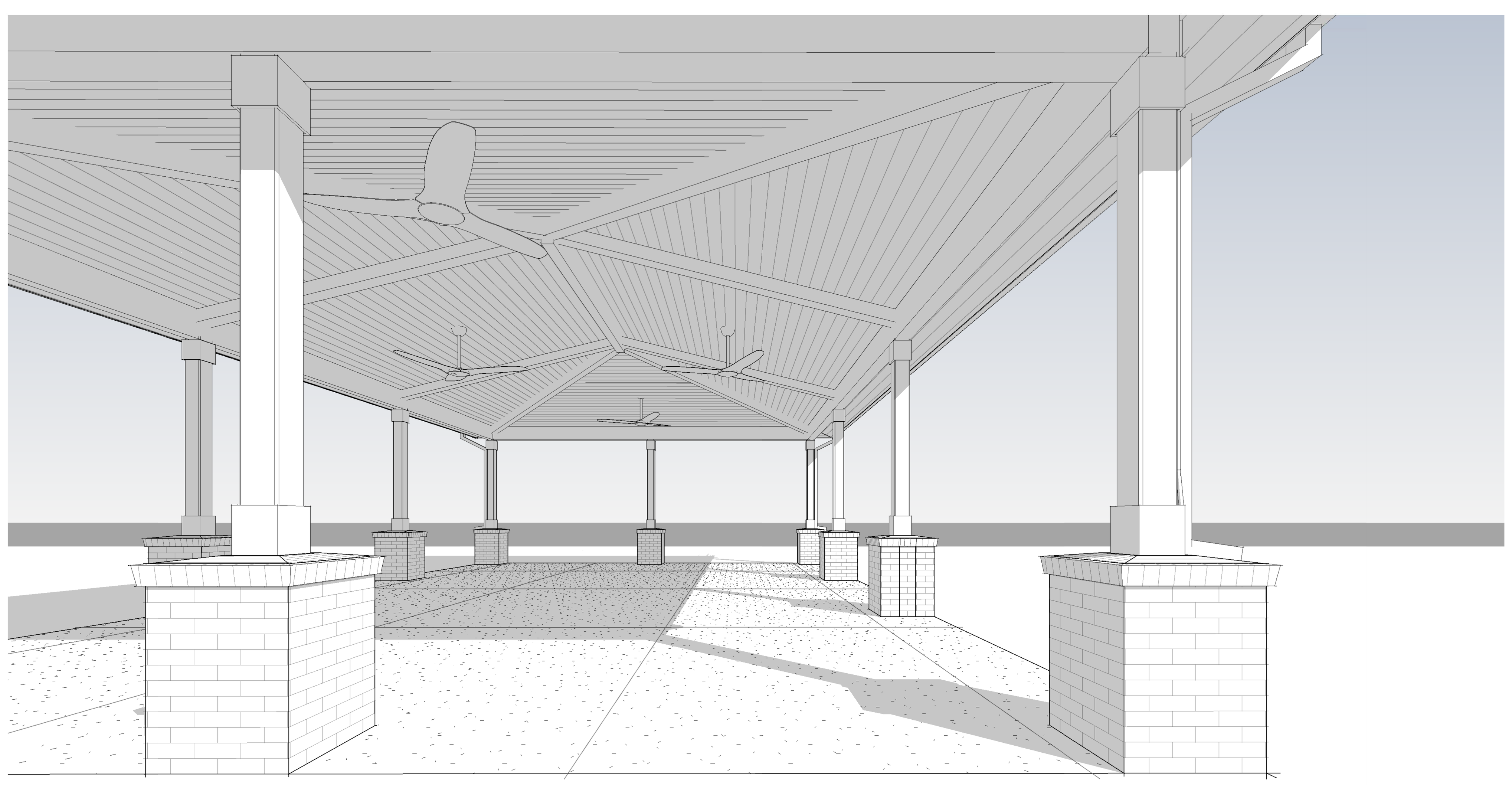


3D VIEW 02

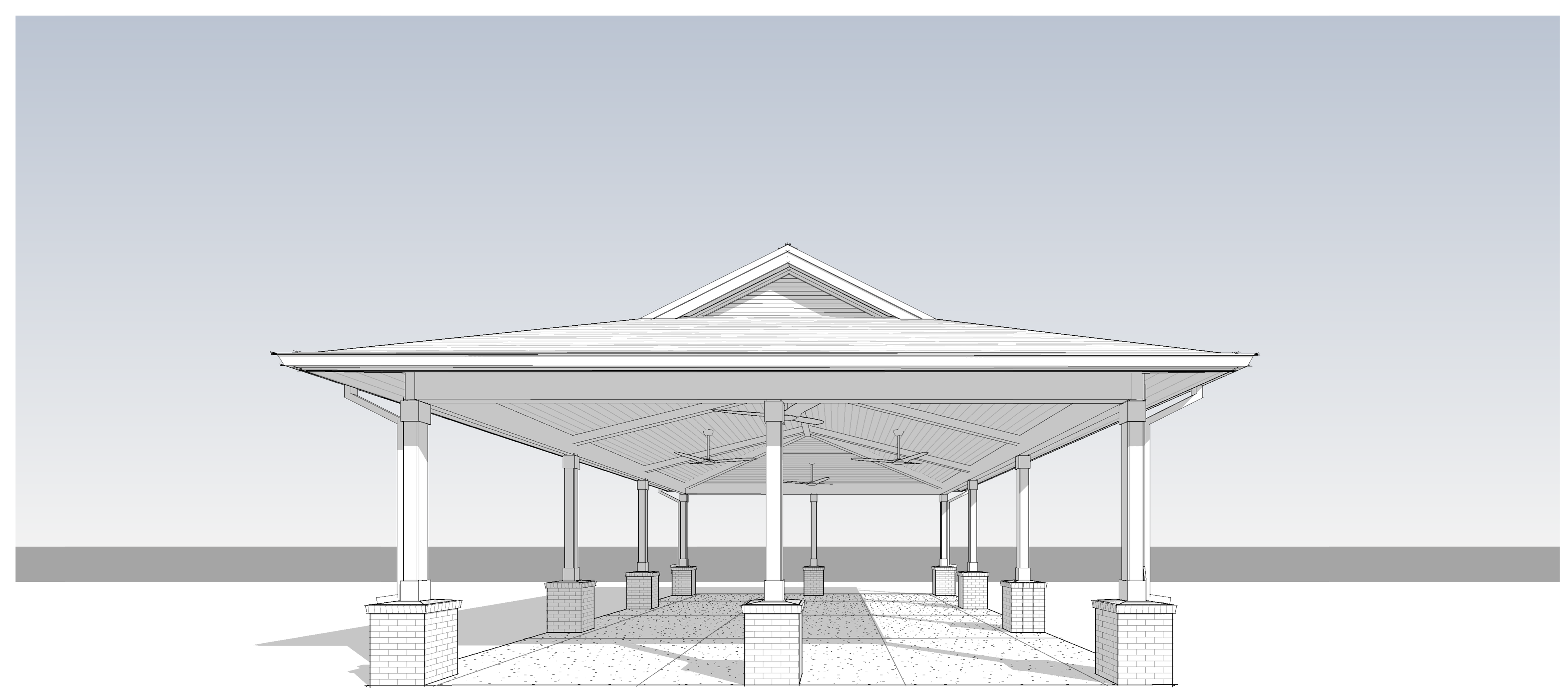
3D VIEW 03



3D VIEW 04



3D VIEW 05



3D VIEW 06

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE
1	Response to Permitting Comments	7/10/2026

STATE OF GEORGIA
STEVEN G. STOWERS
6/17/26
CERTIFICATE NO. 011631
REGISTERED ARCHITECT

FOR CONSTRUCTION

ARCHITECTURE A101
BEAUFORT - SAVANNAH
518 Martin Luther King, Jr. Boulevard | Savannah, Georgia
www.A101.Design | 912.306.0945

3D VIEWS

2026.04
06/17/2026
Drawn By CK
Checked By SGS

G100

7/16/2026 4:31:56 PM

STRUCTURAL DESIGN CRITERIA

BUILDING CODE
2024 INTERNATIONAL BUILDING CODE (IBC)

RISK CATEGORY
RISK CATEGORY = II

GRAVITY LOADS

DESIGN DEAD LOAD TABLE	
CONSTRUCTION	TOTAL DEAD LOAD
ROOF	20 PSF

ROOF LIVE LOAD TABLE		
ROOF TYPE	UNIFORM LIVE LOADING	CONCENTRATED LIVE LOADING
ORDINARY FLAT AND PITCHED ROOF	20 PSF	300 LBS

ROOF SNOW LOADS
GROUND SNOW LOAD: p_g = 6 PSF

WIND LOADS
ULTIMATE DESIGN WIND SPEED: V_{ult} = 131 MPH
NOMINAL DESIGN WIND SPEED: V_{asd} = 102 MPH
RISK CATEGORY = II
WIND EXPOSURE = C
INTERNAL PRESSURE COEFFICIENT: (GCpi) = 0.00 (OPEN)
EDGE DISTANCE PARAMETER: a = 3.0 FT (SEE DIAGRAM)
COMPONENTS AND CLADDING ULTIMATE WIND PRESSURES = (SEE DIAGRAM)

EARTHQUAKE DESIGN DATA

SITE CLASS = D
S_S = 0.420g
S_1 = 0.120g
S_{DS} = 0.300g
S_{D1} = 0.160g
T_L = 8 sec
SEISMIC DESIGN CATEGORY = B (PERMITTED FROM S_{DS} ALONE PER ASCE7-22 SECTION 11.6)

SEISMIC FORCE RESISTING SYSTEM					
SEISMIC FORCE RESISTING SYSTEM	DETAILING SECTION	R	Ω_0	C_d	h_n LIMIT
G6. Timber Frames	14.5	1 ½	1 ½	1 ½	SDC B = 35

RESPONSE MODIFICATION COEFFICIENT, R = 1½
ANALYSIS PROCEDURE UTILIZED = EQUIVALENT LATERAL FORCE PROCEDURE (ASCE 7-22 12.8)
SEISMIC RESPONSE COEFFICIENT, C_s = 0.091
SEISMIC BASE SHEAR, V = 3.9 KIPS

GEOTECHNICAL INFORMATION

PROJECT GEOTECHNICAL REPORT = PRESUMPTIVE VALUES PER IBC 2024 TABLE 1806.2
ALLOWABLE VERTICAL BEARING PRESSURE = 1,500 PSF
ALLOWABLE LATERAL BEARING PRESSURE = 100 PSF/FT BELOW NATURAL GRADE
FOOTING-SOIL COEFFICIENT OF FRICTION = 0.10

FLOOD DESIGN DATA

FLOOD ZONE = X

SPECIAL LOADS

NOT APPLICABLE

REQUIRED SUBMITTALS		
SECTION	SUBMITTAL TYPE	SUBMITTAL DESCRIPTION
03 - CONCRETE		
033000: CAST-IN-PLACE CONCRETE	ACTION	a. PRODUCT DATA
		b. DESIGN MIXTURES
		c. STEEL REINFORCEMENT SHOP DRAWINGS
	INFORMATIONAL	a. MATERIAL CERTIFICATES
		b. MATERIAL TEST REPORTS
		c. FORMWORK SHOP DRAWINGS
		d. FLOOR SURFACE FLATNESS/LEVELNESS MEASUREMENTS
06 - WOOD, PLASTICS, COMPOSITES		
061000: ROUGH CARPENTRY	ACTION	a. PRODUCT DATA
	INFORMATIONAL	a. MATERIAL CERTIFICATES
		b. EVALUATION REPORTS
061600: SHEATHING	ACTION	a. PRODUCT DATA
	INFORMATIONAL	a. EVALUATION REPORTS
061753: SHOP-FABRICATED WOOD TRUSSES	ACTION	a. PRODUCT DATA
		b. SHOP DRAWINGS
		c. DELEGATED-DESIGN SUBMITTAL
	INFORMATIONAL	a. PRODUCT CERTIFICATES
		b. EVALUATION REPORTS

GENERAL REQUIREMENTS

1. THE INTENT OF THESE DRAWINGS IS TO SHOW ALL ITEMS NECESSARY TO COMPLETE THE STRUCTURE. FOR ITEMS, METHODS AND/OR MATERIALS NOT SHOWN, THE MINIMUM REQUIREMENTS OF THE 2024 INTERNATIONAL BUILDING CODE SHALL GOVERN, AS AMENDED BY THE STATE AND LOCAL GOVERNING AGENCIES OF THE PROJECT LOCATION.
2. ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO BIDDING AND/OR THE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO BIDDING AND/OR START OF CONSTRUCTION SO THAT A CLARIFICATION CAN BE PROVIDED.
3. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE OWNER, ARCHITECT OR ENGINEER.
4. DRAWINGS ARE NOT TO BE SCALED. WRITTEN DIMENSIONS SHALL GOVERN CONSTRUCTION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS, SEE THE ARCHITECTURAL DRAWINGS.
5. THE CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DOCUMENTS PRIOR TO BIDDING AND/OR START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND THE ENGINEER SO THAT CLARIFICATION CAN BE PROVIDED. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING REQUIREMENTS OF OTHER DISCIPLINES INTO SHOP DRAWINGS AND CONSTRUCTION ACTIVITIES.
6. THE CONTRACTOR SHALL VISIT THE SITE PRIOR TO BID TO ASCERTAIN CONDITIONS WHICH MAY ADVERSELY AFFECT THE WORK OR COST THEREOF AND SHALL NOTIFY THE ARCHITECT AND ENGINEER IN WRITING PRIOR TO SUBMITTING BIDS.
7. THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OR SEQUENCE OF CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR AND PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT ARE NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES AND SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS). THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND IMPLEMENTATION OF ALL SCAFFOLDING, BRACING AND SHORING.
8. WHERE A SECTION, DETAIL, TYPICAL DETAIL, TYPICAL SECTION, OR PLAN NOTE IS SHOWN FOR ONE CONDITION, IT SHALL APPLY TO ALL LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
9. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES, AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.
10. ANY DELEGATED ENGINEERING DESIGN TO BE PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL MEET THE CRITERIA HEREIN AND SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT .
11. ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE, AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW.
12. NO CHANGE IN SIZE OR DIMENSION OF ANY STRUCTURAL MEMBER SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER OF RECORD. NO OPENING SHALL BE MADE IN ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER OF RECORD, UNLESS SPECIFICALLY DETAILED HEREIN .
13. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION, OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
14. NO STRUCTURAL MEMBER SHALL BE CUT OR NOTCHED OR OTHERWISE REDUCED IN STRENGTH UNLESS APPROVED BY THE STRUCTURAL ENGINEER.
15. DISSIMILAR METALS MUST BE SEPARATED BY A COATING SUCH AS ECK CORROSION COATING OR APPROVED EQUIVALENT OR NEOPRENE GASKET MATERIAL TO PREVENT GALVANIC ACTION.
16. ELECTRONIC DRAWING FILES WILL NOT BE PROVIDED TO THE CONTRACTOR FOR USE IN SHOP DRAWING PREPARATION. ELECTRONIC DRAWING FILES, CONSISTING OF PLAN SHEETS ONLY, MAY BE PROVIDED TO THE CONTRACTOR FOR INFORMATION PURPOSES ONLY (NOT FOR CONSTRUCTION, COST ESTIMATES, OR FABRICATION) UPON RECEIPT OF A DIGITAL DATA TRANSFER AGREEMENT, ALONG WITH A SERVICE FEE PAID BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER OF RECORD.
17. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE DESIGN PROFESSIONAL DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE DESIGN PROFESSIONAL. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATIONS OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS.
18. STRUCTURAL DESIGN PROFESSIONAL IS NOT RESPONSIBLE FOR THE DESIGN OF STEEL STAIRS, HANDRAILS, CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED METAL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
19. SUPPORTS FOR MOBILE CRANES AND FOUNDATIONS FOR TOWER CRANES OR HOISTS, IF APPLICABLE, SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT AND RETAINED BY THE GENERAL CONTRACTOR. DESIGN CALCULATIONS AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND ENGINEER FOR RECORD ONLY.
20. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND DETAILS OF ALL EXTERIOR WALKS, CANOPIES, RAMPS, RAMP WALLS, AND ENTRANCE SLABS NOT DETAILED HEREIN.
21. COORDINATE FLOOR SLAB LAYOUT WITH ARCHITECTURAL DRAWINGS FOR EXACT LIMITS AND DEPRESSIONS FOR AREAS TO RECEIVE ARCHITECTURAL FLOOR FINISHES. COORDINATE FLOOR JOINTS AT DOORS WITH ARCHITECTURAL DOOR DETAILS.
22. NO SPECIFIC PROVISIONS HAVE BEEN MADE FOR HORIZONTAL OR VERTICAL ADDITIONS.

SHOP DRAWINGS AND DEFERRED DOCUMENTS

1. STRUCTURAL DRAWINGS INDICATE TYPICAL AND CERTAIN SPECIFIC CONDITIONS ONLY. SHOP DRAWINGS SHALL DETAIL ALL CONDITIONS IN ACCORDANCE WITH SPECIFIED STANDARDS AND THE SPECIFIC REQUIREMENTS OF THIS PROJECT AS INDICATED IN THE CONSTRUCTION DOCUMENTS.
2. THE GENERAL CONTRACTOR SHALL SUBMIT, AS REQUIRED, PRINTS OR ELECTRONIC COPIES, AS DIRECTED, OF SHOP DRAWINGS FOR ALL FABRICATED MATERIALS TO ARCHITECT FOR REVIEW.
3. REVIEW OF SHOP DRAWINGS BY THE ARCHITECT/STRUCTURAL ENGINEER OF RECORD DOES NOT RELIEVE THE GENERAL CONTRACTOR OF THE SOLE RESPONSIBILITY FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THOSE SHOP DRAWINGS.
4. REPRODUCTION/DUPLICATION OF THE STRUCTURAL DRAWINGS FOR USE IN THE PRODUCTION OF SHOP DRAWINGS IS PROHIBITED, UNLESS NOTED OTHERWISE. IN THE EVENT THAT THE GENERAL CONTRACTOR OR SUBCONTRACTOR ELECTS TO PRODUCE SHOP DRAWINGS BY COPYING ELECTRONIC OR PAPER COPIES OF THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL REQUEST FROM THE STRUCTURAL ENGINEER OF RECORD A SHOP DRAWING WAIVER ALONG WITH THE SPECIFIC SHEETS REQUIRED. SIGNATURE OF THE WAIVER BY THE GENERAL CONTRACTOR, ALONG WITH PAYMENT OF A FEE TO THE STRUCTURAL ENGINEER OF RECORD WILL BE REQUIRED. THE GENERAL CONTRACTOR SHALL CONTINUE TO ASSUME RESPONSIBILITY FOR ERRORS, OMISSIONS AND COORDINATION REQUIRED FOR SHOP DRAWING PRODUCTION, REGARDLESS OF THE USE OF COPIES OF THE STRUCTURAL DRAWINGS FOR SHOP DRAWING PRODUCTION.
5. THE STRUCTURAL ENGINEER OF RECORD HAS DELEGATED THE DESIGN OF SYSTEMS NOT SHOWN IN THE STRUCTURAL DRAWINGS. SUCH SYSTEMS SHALL BE CONSIDERED DEFERRED SUBMITTALS.
6. DEFERRED SUBMITTALS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER (STRUCTURAL ENGINEER WHERE REQUIRED BY JURISDICTION) LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. SUBMITTALS SHALL CLEARLY INDICATE ALL LOADS AND REACTIONS TO BE IMPOSED ON THE PRIMARY STRUCTURE. SUBMITTALS SHALL INCLUDE ALL COMPONENTS AND CONNECTIONS. WHERE CONTRACT DOCUMENTS INDICATE MEMBER OR CONNECTION QUALITIES AND/OR QUANTITIES, THESE SHALL BE CONSIDERED A MINIMUM AND REMAIN THE DESIGN RESPONSIBILITY OF THE COMPONENT ENGINEER TO INCREASE AS REQUIRED TO SATISFY LOAD AND DEFLECTION REQUIREMENTS. DEFERRED SUBMITTALS SHALL INCLUDE CALCULATIONS. BOTH THE CALCULATIONS AND THE SHOP DRAWINGS OF THE DEFERRED SUBMITTAL SHALL BE SEALED PRIOR TO SUBMISSION.
7. DEFERRED SUBMITTALS SHALL BE SUBMITTED TO THE LOCAL AUTHORITY HAVING JURISDICTION AFTER APPROVAL BY THE CONTRACTOR.
8. THE FOLLOWING SHALL BE CONSIDERED DEFERRED SUBMITTALS ON THIS PROJECT (REFER TO SPECIFICATIONS FOR ADDITIONAL INFORMATION):
   A. SEISMIC DESIGN AND ANCHORAGE OF NON-STRUCTURAL COMPONENTS
   B. SHOP FABRICATED WOOD TRUSSES
9. COMPONENTS ATTACHED TO THE BUILDING STRUCTURE (GLAZING, CLADDING, STAIRS, ETC.) SHALL BE DESIGNED AND DETAILED TO ACCOMMODATE AN INELASTIC SEISMIC STORY DRIFT OF 0.02H, WHERE H IS THE STORY HEIGHT IN INCHES. MORE REFINED STORY DRIFTS CAN BE PROVIDED UPON REQUEST.

SLABS ON GRADE

1. UNLESS NOTED OTHERWISE, ALL SLABS ON GRADE SHALL BE MINIMUM 4" THICK.
2. ALL SLABS ON GRADE SHALL BEAR ON SUBGRADE COMPACTED TO MINIMUM 95% ASTM D-1557 UNLESS MORE STRINGENT REQUIREMENTS ARE INDICATED IN PROJECT SPECIFICATIONS OR GEOTECHNICAL REPORT.
3. CAPILLARY BARRIER BELOW SLABS ON GRADE SHALL CONSIST OF 4" COMPACTED CLEAN SAND FILL WITH MINIMUM #5 FINES (MAXIMUM 5% PASSING #200 SIEVE) OR 4" LAYER OF #57 OR #89 STONE.
4. THE CONTRACTOR SHALL COORDINATE ALL LIMITS AND DEPTHS OF DEPRESSIONS FOR FLOOR FINISHES WITH ARCHITECTURAL DRAWINGS AND SCHEDULES. LIMITS SHOWN ON STRUCTURAL DRAWINGS ARE SCHEMATIC.
5. WELDED WIRE FABRIC, WHERE SPECIFIED, SHALL BE PLACED IN FLAT SHEETS (NO ROLLS) AT A CONSISTENT DEPTH OF 1"-2" FROM TOP OF SLAB. WELDED WIRE FABRIC SHALL BE PROPERLY SUPPORTED BY PREMANUFACTURED PLASTIC CHAIRS SPACED AT 48" O.C. MAX EACH WAY. PROVIDE CHAIRS 8" MAX FROM SLAB EDGES AND METAL KEY JOINTS. CONCRETE BRICKS ARE NOT PERMITTED WITHIN SLABS ON GRADE.
6. THE USE OF POLYPROPYLENE FIBERS IN LIEU OF WELDED WIRE FABRIC IS PROHIBITED WITHOUT THE WRITTEN AUTHORIZATION OF THE ENGINEER.
7. PLUMBING AND ELECTRICAL CONDUITS SHALL BE PLACED BELOW THE SLAB AND NOT WITHIN THE SLAB. VERTICAL PENETRATIONS ARE ALLOWED.
8. THE FINISH TOLERANCE OF ALL SLABS SHALL BE IN ACCORDANCE WITH ACI 301, TYPE A.
9. SLABS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE FOLLOWING FLATNESS / LEVELNESS REQUIREMENTS:
   A. FF = 35
   B. FL = 25
10. FF AND FL TOLERANCES SHALL BE TESTED IN ACCORDANCE WITH ASTM E 1155. ACTUAL OVERALL F-NUMBERS SHALL BE CALCULATED USING THE INFERIOR / SUPERIOR AREA METHOD. ALL FLOOR TOLERANCE MEASUREMENTS SHALL BE MADE WITHIN 48 HOURS AFTER SLAB INSTALLATION. IN ALL CASES, TOLERANCE MEASUREMENTS SHALL PRECEDE THE REMOVAL OF SHORES AND FORMS. RESULTS OF ALL FLOOR PROFILE TESTS INCLUDING A RUNNING TABULATION OF THE OVERALL FF AND FL VALUES FOR ALL OF THE RANDOM TRAFFIC SLABS INSTALLED TO DATE SHALL BE PROVIDED TO THE CONTRACTOR WITHIN 72 HOURS AFTER EACH SLAB INSTALLATION.
11. WALKWAYS AND OTHER EXTERIOR SLABS ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS. SEE THE SITE PLAN AND ARCHITECTURAL DRAWINGS FOR LOCATIONS, DIMENSIONS, ELEVATIONS, JOINTING DETAILS AND FINISH DETAILS.
12. C.J. ON PLANS INDICATES CONSTRUCTION JOINTS OR CONTROL JOINTS. IF NOT SHOWN OTHERWISE, MAINTAIN A MAXIMUM AREA BOUNDED BY THE SLAB CONTROL JOINTS OF 225 SQUARE FEET. THE MAXIMUM SIDE RATIO SHALL BE 1.5:1 AND JOINTS AT REENTRANT SLAB CORNERS.
13. SAW-CUT CONTRACTION JOINTS SHALL BE CUT AS SOON AS THE CONCRETE CAN BE CUT WITHOUT RAVELING. CONVENTIONAL CAW-CUT JOINTS SHOULD BE RUN WITHIN 4-12 HOURS AFTER THE CONCRETE HAS BEEN FINISHED. JOINTS PLACED WITH AN EARLY ENTRY SAW MAY BE CUT 1-4 HOURS AFTER THE SLAB HAS BEEN FINISHED.
14. GC SHALL NOTIFY EOR OF PROPOSED CONSTRUCTION JOINT LOCATIONS PRIOR TO CONCRETE PLACEMENT.
15. SLAB JOINTS SHALL BE FILLED WITH APPROVED MATERIAL. THIS SHOULD TAKE PLACE AS LATE AS POSSIBLE PRIOR TO FILLING, REMOVE ALL DEBRIS FROM THE SLAB JOINTS, THEN FILL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AS FOLLOWS:
   A. CONTROL JOINT SEALANT: EUCLID CHEMICAL DURAL 340 SL OR APPROVED EQUAL
   B. EXPANSION/ISOLATION JOINT SEALANT: EUCLID CHEMICAL EUCOLASTIC 1 SL OR APPROVED EQUAL
16. CONCRETE FINISH FLOORS SHALL HAVE A HARD STEEL TROWELED FINISH UNLESS INDICATED OTHERWISE ON THE DRAWINGS. PLACE, STRIKE OFF, CONSOLIDATE, LEVEL AND FLOAT TO THE PROPER ELEVATION. TROWELING SHALL BEGIN AFTER SURFACE HAS RECEIVED A FLOAT FINISH. THE SLAB DRYING MUST PROCEED NATURALLY AND MUST NOT BE HASTENED BY THE DUSTING ON OF DRY CEMENT OR SAND. LIGHTLY TOOL ALL EDGES AT CONSTRUCTION JOINTS AND EXERCISE CARE THAT SLAB EDGES ARE NOT DEPRESSED ALONG BULKHEADS DURING FINISHING OPERATIONS, PARTICULARLY HAND TROWELING. EXTERIOR SLABS, SIDEWALKS, PADS AND RAMPS SHALL HAVE A LIGHT BROOM FINISH UNLESS INDICATED OTHERWISE ON THE DRAWINGS. PROVIDE STANDARD TROWEL FINISH AT ALL SUB-SLABS.
17. PROVIDE 1/2" PREMOLDED EXPANSION JOINT (P.E.J.) FILLER AROUND PERIMETER OF SLABS WHERE THEY ABUT VERTICAL SURFACES AND AT COLUMN ISOLATION JOINTS AS DETAILED.

FOUNDATIONS

1. IN ABSENCE OF A PROJECT GEOTECHNICAL REPORT, THE FOUNDATION IS DESIGNED BASED UPON PRESUMPTIVE LOAD-BEARING VALUES OF IBC TABLE 1806.2. A GEOTECHNICAL ENGINEER SHALL BE OBTAINED BY THE CONTRACTOR TO VERIFY THE SUITABILITY OF SHALLOW FOUNDATIONS. THE SOIL PRESSURES LISTED BELOW SHALL ALSO BE CONFIRMED BY FIELD TESTING USING A DYNAMIC CONE PENETROMETER TEST (ASTM STP-300) AT EACH COLUMN FOUNDATION EXCAVATION AND 75 FEET MAXIMUM SPACING ON WALL FOOTING AND THICKENED SLAB EXCAVATIONS.
2. ALLOWABLE SOIL BEARING PRESSURES USED FOR FOUNDATION DESIGN:
   A. ALLOWABLE BEARING PRESSURE = 1500 PSF
   B. ALLOWABLE PASSIVE PRESSURE = 200 PCF
4. A GEOTECHNICAL ENGINEER SHALL BE OBTAINED BY THE CONTRACTOR FOR GUIDANCE ON PREPARING THE SUBGRADE TO ADEQUATELY ACHIEVE THE ASSUMED SOIL BEARING PRESSURE NOTED HEREIN. IF UNSUITABLE SUBSURFACE CONDITIONS ARE ENCOUNTERED, CONTACT THE ARCHITECT AND ENGINEER IMMEDIATELY. AT MINIMUM, THE CONTRACTOR SHALL PREPARE THE SUBGRADE AS FOLLOWS:
   4.1. CONTRACTOR SHALL STRIP AND REMOVE ALL VEGETATION, TOPSOIL, ROOTS, AND ORGANIC ZONES FROM THE CONSTRUCTION AREA. THE DEPTH OF STRIPPING SHALL BE REQUIRED TO REMOVE SIGNIFICANT ROOT ZONES, SMALL TREE STUMPS, AND OTHER UNACCEPTABLE MATERIALS, BUT IN NO CASE SHALL IT BE LESS THAN 12 INCHES.
   4.2. AFTER TOPSOILS, ETC. WITHIN AND TO A POINT 10' OUTSIDE THE BUILDING CONSTRUCTION AREA HAVE BEEN REMOVED FROM THE SITE, THE UPPER 24 INCHES OF EXPOSED SOILS SHALL BE COMPACTED TO A MINIMUM DENSITY OF 95% MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D-1557) BY PROOFROLLING WITH A FULLY LOADED PNEUMATIC TIRED TANDEM AXLE DUMP TRUCK CAPABLE OF TRANSFERRING A LOAD OF 10 TO 20 TONS BY OVERLAPPING PASSES. A MINIMUM OF 8 COMPLETE PASSES SHALL BE MADE WITHIN THE BUILDING AREA.
   4.3. PROOFROLLING SHALL BE PERFORMED UNDER THE OBSERVATION OF AN APPROVED TESTING LABORATORY SUPERVISED BY A GEOTECHNICAL ENGINEER. UNDERCUT, BACKFILL, AND COMPACT AREAS WHICH PUMP, DEFLECT, OR RUT EXCESSIVE OR WHICH DO NOT STABILIZE AFTER SUCCESSIVE PASSES OF PROOFROLLING EQUIPMENT.
   4.4. AFTER COMPLETION OF DENSIFICATION OF EXISTING SOILS, PLACE STRUCTURAL FILL FOR BUILDING AND PAVEMENT ARES IN THIN (8" TO 10") LIFTS AND COMPACT TO A MINIMUM DENSITY OF 95% MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D-1557). MATERIAL USED AS STRUCTURAL FILL SHALL BE NON-PLASTIC GRANULAR MATERIAL CONTAINING LESS THAN 15% FINES PASSING THROUGH THE NO. 200 SIEVE AND FREE OF ORGANICS. BOULDERS, OR OTHER DELETERIOUS MATERIALS.
   4.5. ANY AREA THAT WILL BE USED FOR PARKING AND ALL DRIVEWAYS SHALL HAVE A MINIMUM OF 24" CLEAN SAND SUBGRADE COMPACTED TO A MINIMUM DENSITY OF 95% MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D-1557). PROVISIONS SHALL BE MADE FOR SUBGRADE TO DRAIN AND NOT BECOME SATURATED BY SURFACE WATER OR GROUND WATER TABLE.
5. ALL FOUNDATIONS SHALL BE PLACED ON COMPACTED SUBGRADE.
6. THE BOTTOM OF ALL EXTERIOR FOUNDATIONS SHALL BE A MINIMUM OF 12 INCHES BELOW FINISHED GRADE UNLESS NOTED OTHERWISE.
7. REMOVE ALL WATER SOFTENED SOILS FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE. FILL REMAINING VOIDS WITH ADDITIONAL CONCRETE.
8. ALL FOUNDATION REINFORCEMENT SHALL BE PROPERLY TIED IN PLACE PRIOR TO PLACEMENT OF CONCRETE. BOTTOM REINFORCING SHALL BE SUPPORTED WITH WHOLE CONCRETE BRICKS OR PREFABRICATED ALL PLASTIC CHAIR SUPPORTS AT 48" O.C. MAX. BAR SUPPORTS SHALL BE POSITIONED TO MAINTAIN NO LESS THAN 3" CLEAR TO BOTTOM OF LOWEST REINFORCING BAR.
9. NO FOOTINGS SHALL BEAR ON ROCK. UNDERCUT ROCK A MINIMUM OF 24 INCHES BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL IN ACCORDANCE WITH PROJECT SPECIFICATIONS.
10. WHERE FINISHED GRADES DIFFER ON OPPOSITE SIDES OF FOUNDATION WALLS, PROVIDE TEMPORARY BRACING TO PREVENT LATERAL MOVEMENT UNTIL ALL ADJACENT FILL, COMPACTION, FLOOR SLABS, AND FRAMING AT NEXT LEVEL HAVE BEEN COMPLETED.
11. WHERE GRAVITY PLUMBING LINES OCCUR BELOW TOP OF WALL FOOTING, STEP FOOTING DOWN TO PROVIDE CLEARANCES INDICATED ON DETAIL "WALL FOOTING DETAILS - INTERFERENCE OFFSET AT GRAVITY SEWER" UNLESS NOTED OTHERWISE. COORDINATE WITH PLUMBING DRAWINGS FOR LOCATIONS, SIZES, AND INVERTS.
12. ALL FOOTINGS SHALL BE CENTERED ABOUT COLUMN LINES UNLESS NOTED OTHERWISE.
13. WHERE VERTICAL STEPS IN WALL FOOTINGS SHOWN ON FOUNDATION PLANS, THEY SHALL BE A MAXIMUM OF 2'-0" HIGH SPACED NO CLOSER THAN 4'-0" O.C.
14. CONSTRUCTION JOINTS IN WALL FOOTINGS SHALL BE FORMED VERTICALLY WITH MINIMUM 2'-0" LAP HORIZONTAL REINFORCING.
15. SUPPORTS FOR MOBILE CRANES AND FOUNDATIONS FOR TOWER CRANES OR HOISTS, IF APPLICABLE, SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT AND RETAINED BY THE GENERAL CONTRACTOR. DESIGN CALCULATIONS AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND ENGINEER FOR RECORD ONLY.

CAST-IN-PLACE CONCRETE

1. ALL CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL COMPLY WITH THE MOST RECENT EDITION OF THE FOLLOWING ACI PUBLICATIONS:
   A. ACI 301 - GENERAL CONSTRUCTION REQUIREMENTS
   B. ACI 117 - TOLERANCES FOR CONCRETE CONSTRUCTION
2. CONCRETE SHALL BE NORMAL-WEIGHT CONCRETE (145 PCF) WITH MIXES MEETING THE FOLLOWING CRITERIA:
   A. FOUNDATION ELEMENTS & SLAB ON GRADE
      1. MINIMUM 28-DAY COMPRESSIVE STRENGTH = 4000 PSI
      2. COARSE AGGREGATE SIZE = #57 STONE
      3. MAXIMUM WATER-TO-CEMENTITOUS MATERIALS RATIO = 0.50
      4. SLUMP LIMIT = 5 INCHES (±1 INCH)
      5. AIR CONTENT = 4.5% (+/-1.5%)
      6. ACCEPTABLE CEMENTIOUS MATERIALS:
   A. PORTLAND CEMENT - ASTM C 150, TYPE II
   B. FLY ASH - ASTM C 618
   C. SLAG CEMENT - ASTM C989
   D. BLENDED HYDRAULIC CEMENT - ASTM C 595, TYPE IS OR TYPE IP
3. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT PERMITTED.
4. ALL EXPOSED CONCRETE EDGES SHALL HAVE A 3/4-INCH CHAMFER.
5. OPENINGS, POCKETS, ETC., LARGER THAN 6" SHALL NOT BE PLACED IN CONCRETE SLABS, DECKS, WALLS, UNLESS SPECIALLY DETAILED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW OPENINGS, POCKETS, ETC., LARGER THAN 6" NOT SHOWN ON THE STRUCTURAL DRAWINGS, BUT WHICH ARE LOCATED IN STRUCTURAL MEMBERS. FOR ANY FURTHER RESTRICTIONS ON OPENINGS IN STRUCTURAL ELEMENTS, SEE APPLICABLE SECTIONS BELOW.
6. UNLESS NOTED OR DETAILED OTHERWISE IN THE DRAWINGS, CAST IN PLACE CONCRETE SHALL HAVE THE FOLLOWING TRIM STEEL ADDED AROUND ALL OPENINGS: TWO (2) - #5 BARS (LENGTH OF BARS = LENGTH OF OPENING + 4'-0") ALONG EACH SIDE OF OPENING AND TWO (2) - #5 X 5'-0" DIAGONALLY AT EACH CORNER.
7. PIPES LARGER THAN 1 ½" DIAMETER SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY APPROVED. NO CONDUITS SHALL BE PLACED IN CONCRETE FILL OVER METAL DECKING.
8. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS, OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
9. DEFECTIVE AREAS IN CONCRETE, INCLUDING BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.016 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE ENGINEER OF RECORD.
10. REQUIRED CAST-IN-PLACE CONCRETE SUBMITTALS TO ENGINEER:
   A. PRODUCT DATA - SUBMIT TECHNICAL PRODUCT DATA FOR ANY ADMIXTURES OR CONCRETE-RELATED CONSTRUCTION PRODUCTS.
   B. DESIGN MIXTURES - THE FOLLOWING ITEMS ARE REQUIRED:
      1. MIX IDENTIFICATION BY MEANS OF CLASS OR LOCATION WHERE MIX WILL BE USED.
      2. STRENGTH OF CONCRETE.
      3. TARGET SLUMP, WATER-TO-CEMENT RATIO, DENSITY, AND AIR CONTENT.
      4. LIST OF ALL MATERIALS, ADMIXTURES, AND ADDITIVES ALONG WITH THEIR PROPORTIONS.
      5. NOMINAL MAXIMUM AGGREGATE SIZE AND COMBINED AGGREGATE GRADATION.
      6. CALCULATIONS AND TEST RESULTS REQUIRED BY ACI 318-14 CHAPTER 26
      7. TEST RESULTS OF TOTAL CHLORIDE CONTENT.
      8. INFORMATION ON CONCRETE MATERIALS AS PER ACI 301-14 SECTION 26.4
      9. TEST RESULTS PER ASTM C33, INCLUDING THE CLEANNESS VALUE, SAND EQUIVALENT, AND ALKALI-SILICA REACTIVITY (ASR) POTENTIAL AND MITIGATION, IF REQUIRED.
      10. MILL CERTIFICATE FOR THE CEMENT INDICATING THE SOURCE OF THE CEMENT AND COMPLIANCE WITH THE PROJECT SPECIFICATION.
      11. MILL ANALYSIS FOR SUPPLEMENTARY CEMENTITIOUS MATERIALS (INCLUDING FLY ASH AND SLAG CEMENT) AND AGGREGATES FROM THE MANUFACTURER.
      12. CERTIFICATION BY THE MANUFACTURERS THAT THE ADMIXTURES CONFORM TO THE SPECIFIED STANDARDS.
      13. WHETHER MIX IS APPROPRIATE FOR PUMPING.
      14. THERMAL CONTROL PLAN, INCLUDING HOT WEATHER AND COLD WEATHER PLACEMENT.
   C. STEEL REINFORCEMENT SHOP DRAWINGS - PLACING DRAWINGS THAT DETAIL FABRICATION, BENDING, AND PLACEMENT OF REINFORCEMENT.
   D. 28-DAY CONCRETE ACCEPTANCE TEST REPORT AS REQUIRED BY ACI 318-14 SECTION 26.13.2

ABBREVIATIONS

@	AT	LLH	LONG LEG HORIZONTAL
(#)	QUANTITY	LLO	LONG LEG OUTSTANDING
AB	ANCHOR BOLT	LLV	LONG LEG VERTICAL
ABV.	ABOVE	LONG.	LONGITUDINAL
ADD'L	ADDITIONAL	LSL	LAMINATED STRAND LUMBER
A.R.	ANCHOR ROD	LVL	LAMINATED VENEER LUMBER
ARCH.	ARCHITECTURAL	MAX.	MAXIMUM
B/	BOTTOM OF	MIN.	MINIMUM
BM	BEAM	MIR.	MIRROR
BP	BASE PLATE	N.I.C.	NOT IN CONTRACT
BPL	BENT PLATE	O.C.	ON CENTER
C	CHANNEL	OWWT	OPEN WEB WOOD TRUSS
CIP	CAST IN PLACE	PC.	PIECE
CJ	CONTROL JOINT	P.E.J.	PLASTIC EXPANSION JOINT
CL	CENTER LINE	PL	PLATE
CMU	CONCRETE MASONRY UNIT	PLF	POUNDS PER LINEAR FOOT
CLR.	CLEAR	PSF	POUNDS PER SQUARE FOOT
COL	COLUMN	PSL	PARALLEL STRAND LUMBER
CONC.	CONCRETE	P.T.	PRESSURE TREATED
CONT.	CONTINUOUS	REINF.	REINFORCEMENT
COORD.	COORDINATE	REQ'D.	REQUIRED
D.	DEPTH	SCHED.	SCHEDULE
DBL.	DOUBLE	SDS	SELF-DRILLING SCREW
DEV.	DEVELOPMENT	SIM.	SIMILAR
DIA. or Ø	DIAMETER	SJ	SAW JOINT
DL	DEAD LOAD	SLH	SHORT LEG HORIZONTAL
EA.	EACH	SLO	SHORT LEG OUTSTANDING
E.F.	EACH FACE	SLV	SHORT LEG VERTICAL
EL.	ELEVATION	SPF	SPRUCE-PINE-FIR
ELEV.	ELEVATION	SQ.FT.	SQUARE FEET
E.S.	EACH SIDE	STRUCT.	STRUCTURAL
E.W.	EACH WAY	SYP	SOUTHERN YELLOW PINE
EXIST.	EXISTING	T&B	TOP & BOTTOM
FFE	FINISHED FLOOR ELEVATION	T/	TOP OF
FOUND.	FOUNDATION	TBE	TRUSS BEARING ELEVATION
FT. or '	FEET	TD	TURN DOWN
FTG.	FOOTING	TJI	iLEVEL TRUS JOIST
GA.	GAUGE	TOF	TOP OF FOOTING
GALV.	GALVANIZED	TOJ	TOP OF JOIST
GB	GRADE BEAM	TOS	TOP OF STEEL
HDR	HEADER	TPL.	TRIPLE
HORIZ.	HORIZONTAL	TRANSV.	TRANSVERSE
HSS	HOLLOW STRUCTURAL SECTION	TS	THICKENED SLAB
IN. or "	INCH	TYP.	TYPICAL
JBE	JOIST BEARING ELEVATION	U.N.O.	UNLESS NOTED OTHERWISE
JST.	JOIST	VERT.	VERTICAL
K	KIPS	V.I.F.	VERIFY IN FIELD
KSI	KIPS PER SQUARE INCH	w/	WITH
L	ANGLE	W.	WIDTH
L.	LENGTH	WF	WIDE FLANGE
LBS.	POUNDS	WL	WIND LOAD
LG.	LONG	WWF	WELDED WIRE FABRIC
LL	LIVE LOAD		

PLAN LEGEND & SYMBOLS

VIEW SCALE View Name
VIEW NAME, MARK, AND SCALE

DETAIL # # SHEET # S### SIM
SECTION CUT

/ S###
VIEW REFERENCE: LETTER & SHEET NUMBER

1
REVISION MARK

DETAIL # # SHEET # S### SIM
PLAN/SECTION REFERENCE

BEAM TYPE @ ##" O.C.
FRAMING NOTE: TYPE AND SPACING

— STRUCTURAL GRID LINE TO FACE OF CMU OR WALL CENTERLINE PER PLAN

POST SIZE — WOOD POST SCHEDULE: PT88 = 8x8 SOLID PRESSURE TREATED

WOOD POST - SIZE AS NOTED ON OTHER PLANS

FOOTING EXTENTS

WALL (ABOVE) - TYPE AS NOTED

F#.# — INDICATES WALL OR COLUMN FOOTING. SEE SCHEDULE

[±FT-IN] — INDICATES ELEVATION OF TOP OF FOOTING (FROM 0'-0")

BEAM SIZE — BEAM SIZE DESIGNATION; BEAM EXTENTS
[±FT-IN] — CHANGE IN TOS ELEVATION FROM TYP. (IF DIFFERENT)

RAFTER OR JOIST - SIZE AND SPACING AS NOTED

CJ — SLAB CONTROL JOINT SEE C / S301 FOR CONTROL & CONSTRUCTION JOINT DETAILS

#:# — SLOPE MARKER RISE:RUN

±FT-IN TARGET — ELEVATION INDICATOR RELATIVE TO GROUND FFE @ 0'-0" ELEVATION TARGET

STRUCTURAL SHEET LIST

SHEET NUMBER	SHEET NAME	CURRENT REVISION
S001	GENERAL NOTES, SHEET LIST & LEGEND	
S002	GENERAL NOTES	
S003	C&C DIAGRAM	
S004	SPECIAL INSPECTIONS FORMS	
S005	SPECIAL INSPECTIONS SCHEDULES	
S101	PLAN VIEWS	
S301	FOUNDATION SECTIONS & TYPICAL DETAILS	
S401	FRAMING SECTIONS & TYPICAL DETAILS	

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE

GEORGIA REGISTERED No. SE000801 STRUCTURAL 06/17/2026 ENGINEER MICHAEL CODY THARPE

FOR PERMIT

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GENERAL NOTES, SHEET LIST & LEGEND

	2026.04
	06/17/2026
Drawn By	GMB
Checked By	SJP

S001

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REINFORCING STEEL

1. FABRICATING, PLACING, AND SUPPORTING REINFORCEMENT SHALL COMPLY WITH CRSI'S "MANUAL OF STANDARD PRACTICE."
2. REINFORCING BARS SHALL BE ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE.
3. REINFORCING BARS IN WELDED CONDITIONS, WHERE PERMITTED, SHALL BE ASTM A 706, DEFORMED.
4. STEEL WELDED-WIRE REINFORCEMENTS SHALL BE ASTM A 1064 WITH 70 KSI MINIMUM YIELD STRENGTH AND SHALL HAVE MINIMUM SIDE AND END LAPS OF 8".
5. NO REINFORCEMENT SHALL BE FLAME-CUT OR BENT IN FIELD WITHOUT GUIDANCE FROM STRUCTURAL ENGINEER.
6. SPLICES SHALL BE CLASS B IN ACCORDANCE WITH ACI 318, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY THE CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE ENGINEER OF RECORD.
7. ALL HORIZONTAL REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS, UNLESS NOTED OTHERWISE.
8. REINFORCING STEEL SHALL HAVE COVER PROTECTION AS FOLLOWS:

CONCRETE COVER PROTECTION TABLE	
CONDITION	MINIMUM COVER
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
WALL PANELS, SLABS, JOISTS	1 INCH
OTHER MEMBERS	1½ INCHES
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS	¾ INCHES
BEAMS, COLUMNS:	
PRIMARY REINFORCEMENT	1½ INCHES
TIES, STIRRUPS, SPIRALS	1 INCH

WOOD STRUCTURAL PANELS

1. ALL WOOD STRUCTURAL PANELS SHALL BE APA TRADEMARKED STRUCTURAL-USE PANELS QUALIFIED AND MANUFACTURED IN ACCORDANCE WITH APA PRP-108 (PERFORMANCE STANDARDS AND QUALIFICATION POLICY FOR STRUCTURAL-USE PANELS) AND U.S. DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARDS PS 1-09 (STRUCTURAL PLYWOOD) AND PS 2-04 (PERFORMANCE STANDARD FOR WOOD-BASED STRUCTURAL-USE PANELS) AND SHALL BE IDENTIFIED BY THE MARK OF AN APPROVED TESTING AND GRADING AGENCY.

WOOD STRUCTURAL PANELS					
USAGE	THICKNESS	CONSTRUCTION	BOND CLASSIFICATION	PANEL GRADE	SPAN RATING
ROOF	$^{15}/_{32}$ IN.	OSB	EXP. 1	SHEATHING	$^{32}/_{16}$

2. ALL FLOOR PANELS SHALL BE TONGUE & GROOVE. FIELD-GLUE USING ADHESIVES MEETING APA SPECIFICATIONS AFG-01, APPLIED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS.
3. INSTALL ALL PANELS AT FLOOR AND ROOF WITH THE LONG DIMENSIONS OF THE PANEL ACROSS SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. STAGGER PANEL END JOINTS. ALLOW 1/8" SPACING AT PANEL ENDS AND EDGES UNLESS OTHERWISE RECOMMENDED BY THE PANEL MANUFACTURER
4. PROVIDE BLOCKING AT UNSUPPORTED PANEL EDGES AS FOLLOWS:
   A. ROOFS - FULLY BLOCKED. WHERE BLOCKING IS NOT SPECIFICALLY REQUIRED FOR THE ROOF SHEATHING, PLY CLIPS OR TONGUE AND GROOVE PLYWOOD SHALL BE USED.
   B. FLOORS - UNBLOCKED
   C. SHEAR WALLS - FULLY BLOCKED
5. ALL NAILS FOR PANEL ATTACHMENT SHALL NOT BE OVERDRIVEN.
6. WHERE EITHER 2" OR 2 1/2" FASTENER SPACINGS ARE USED FOR WOOD STRUCTURAL PANELS USED AT ROOF OR FLOOR, THE FRAMING MEMBER AT THE ADJOINING PANEL SHALL BE 3" NOMINAL WIDTH AND THE NAILS AT PANEL EDGES SHALL BE STAGGERED IN TWO LINES.
7. NAILS AT ABUTTING PANEL EDGES MUST PENETRATE THE SAME PIECE OF FRAMING OR BLOCKING.

SHOP FABRICATED WOOD TRUSSES

1. TRUSSES SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT STATE TO BE IN ACCORDANCE WITH THE NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION (ANSI/TPI 1) AND IN ACCORDANCE WITH THE AMERICAN FOREST & PAPER ASSOCIATION'S NATIONAL DESIGN SPECIFICATION.
2. TRUSSES SHALL BE DESIGNED, FABRICATED, AND ERECTED TO SUPPORT THE FOLLOWING:
   A. TYPICAL GRAVITY LOADS
      1. TOP CHORD DEAD LOAD = 5 PSF
      2. TOP CHORD LIVE LOAD = 20 PSF
      3. BOTTOM CHORD DEAD LOAD = 10 PSF
      4. BOTTOM CHORD LIVE LOAD = 10 PSF
   B. ENVIRONMENTAL LOADS AS DEFINED IN THE STRUCTURAL DESIGN CRITERIA
   C. SPECIAL LOADS AS DEPICTED ON THE PLAN DRAWINGS
   D. DRAG STRUT LOADS AS DEPICTED ON THE PLAN DRAWINGS
3. TRUSSES SHALL BE DESIGNED, FABRICATED, AND ERECTED TO PERFORM WITH THE FOLLOWING DEFLECTION LIMITS:
   A. ROOF TRUSSES: VERTICAL DEFLECTION LESS THAN OR EQUAL TO L/240.
   B. FLOOR TRUSSES: VERTICAL DEFLECTION LESS THAN OR EQUAL TO L/480.
4. WOOD MATERIALS SHALL BE SOUTHERN PINE, DOUGLAS F/R OR LARCH AND SHALL BE KILN DRIED AND USED AT 19% MAXIMUM MOISTURE CONTENT. PROVIDE GRADE NO. 2 OR AS REQUIRED TO SATISFY STRESS REQUIREMENTS.
5. METAL CONNECTOR PLATES SHALL BE MANUFACTURED BY A TRUSS PLATE INSTITUTE (TPI) MEMBER PLATE MANUFACTURER AND SHALL NOT BE LESS THAN 0.036 INCHES THICK (20 GAUGE) AND SHALL MEET OR EXCEED ASTM A653 GRADE 33, AND GALVANIZED COATING SHALL MEET OR EXCEED ASTM A924 COATING DESIGNATION G60.
6. SPECIALTY ENGINEER SHALL DESIGN AND DETAIL THE SUPPORT CONDITIONS AND THE TRUSS-TO-TRUSS CONNECTIONS FOR THE MAXIMUM REACTION FORCES.
7. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR SAFE ERECTION AND PERFORMANCE OF THE TRUSSES. THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91, COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" SHALL BE A MINIMUM REQUIREMENT.
8. THE TRUSS MANUFACTURER SHALL SUBMIT PRODUCT DATA, SHOP DRAWINGS, AND DESIGN CALCULATIONS INCLUDING THE FOLLOWING:
   A. PRODUCT DATA: FOR EACH TYPE OF CONNECTOR AND FASTENERS UTILIZED FOR THE TRUSS INSTALLATION.
   B. SHOP DRAWINGS INCLUDING THE FOLLOWING:
      1. MEMBER LAYOUT, SLOPE, DEPTH, SPAN, NUMBER OF PLIES, AND SPACING
      2. TRUSS MEMBER SIZES, SPECIES AND GRADE
      3. REQUIRED BEARING WIDTHS
      4. DESIGN LOADS AND MAXIMUM REACTIONS
      5. SUPPORT CONNECTIONS AND TRUSS-TO-TRUSS CONNECTIONS
   C. DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER OF THE PROJECT STATE INCLUDING THE FOLLOWING:
      1. REFERENCE CODES AND STANDARDS
      2. DESIGN LOADS
      3. UTILIZATION RATIOS
      4. DEFLECTIONS
      5. MAXIMUM REACTIONS

ROUGH CARPENTRY

1. ALL WOOD FRAMING SHALL COMPLY WITH THE NATIONAL FOREST PRODUCTS ASSOCIATION'S NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS).
2. ALL WOOD FRAMING MEMBERS ARE DESIGNED TO ACT AS A SYSTEM ONCE CONSTRUCTION IS COMPLETE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE SAFETY AND STABILITY OF WOOD FRAMING SYSTEMS (I.E. TEMPORARY BRACING) DURING CONSTRUCTION AS A RESULT OF CONSTRUCTION METHODS AND SEQUENCES.
3. WOOD FRAMING MATERIALS:
   A. DIMENSIONAL LUMBER FOR HORIZONTAL MEMBERS SHALL BE #2 SOUTHERN YELLOW PINE.
   B. LAMINATED VENEER LUMBER (LVL) SHALL BE MICROLLAM LVL GRADE 2.0E MANUFACTURED BY WEYERHAEUSER TRUS JOIST OR APPROVED EQUIVALENT.
   C. PARALLEL STRAND LUMBER (PSL) SHALL BE PARALLAM PSL MANUFACTURERED BY WEYERHAEUSER TRUS JOIST OR APPROVED EQUIVALENT WITH THE FOLLOWING GRADES:
      1. HEADER AND BEAM SIZES = 2.0E GRADE
      2. COLUMN AND POST SIZES = 1.8E GRADE
   D. LAMINATED STRAND LUMBER (LSL) SHALL BE TIMBERSTRAND LSL MANUFACTURERED BY WEYERHAEUSER TRUS JOIST OR APPROVED EQUIVALENT WITH THE FOLLOWING GRADES:
      1. HEADER AND BEAM SIZES = 1.55E GRADE
      2. COLUMN AND POST SIZES = 1.3E GRADE.
   E. WOOD I-JOISTS SHALL BE TJI JOISTS MANUFACTURED BY WEYERHAEUSER TRUST JOIST OR APPROVED EQUIVALENT.
4. ALL WOOD FRAMING MATERIAL SHALL BE SURFACED DRY AND USED AT 19% MAXIMUM MOISTURE CONTENT.
5. ALL LUMBER EXPOSED TO EXTERIOR ENVIRONMENT OR IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED TO A MINIMUM RETENTION OF 0.25 lbs. OF ACQ PER CUBIC FOOT OF WOOD, AND EACH PIECE SHALL BEAR THE THIRD PARTY QUALITY MARK, "ABOVE GRADE USE". ALL LUMBER IN CONTACT WITH THE GROUND SHALL BE PRESSURE TREATED TO A MINIMUM RETENTION OF 0.40 lbs. OF ACQ PER CUBIC FOOT OF WOOD, AND EACH PIECE SHALL BEAR THE THIRD PARTY QUALITY MARK, "GROUND CONTACT USE". REFERENCE STANDARD AWPA C2 AND ASTM D1760 FOR PRESSURE TREATMENT OF TIMBER PRODUCTS.
6. WHERE POSSIBLE ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO ON-SITE FABRICATION SHALL BE BRUSHED WITH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STD. M4).
7. THE CONTRACTOR SHALL CAREFULLY SELECT LUMBER TO BE USED IN LOAD BEARING APPLICATIONS. THE LENGTH OF SPLIT ON THE WIDE FACE OF 2" NOMINAL LOAD BEARING FRAMING SHALL BE LIMITED TO LESS THAN ½ OF THE WIDE FACE DIMENSION. THE LENGTH OF SPLIT ON THE WIDE FACE OF 3" (NOMINAL) AND THICKER LUMBER SHALL BE LIMITED TO ¼ OF THE NARROW FACE DIMENSIONS.
8. LOCATION, NUMBER, AND DIMENSIONS OF FRAMING MEMBERS SHOW GENERAL ARRANGEMENT ONLY. ACTUAL SPANS, SPACINGS, ETC. SHALL BE DETERMINED FROM ARCHITECTURAL DETAILS.
9. SEE ARCHITECTURAL PLANS AND DETAILS FOR EDGE SECTIONS, HEADER AND LINTEL LOCATIONS, AND ALL NON-STRUCTURAL FRAMING AND TRIM.
10. FOR ALL JOISTS AND RAFTERS, PROVIDE FULL-DEPTH BLOCKING AT ENDS, AT MIDSPAN, AND AT A MAXIMUM SPACING OF 8 FEET IN BETWEEN.
11. ALL ENGINEERED WOOD PRODUCTS SHALL BE BRIDGED, BLOCKED, AND BRACED IN CONFORMANCE WITH THE MANUFACTURERS RECOMMENDATION.
12. LOAD BEARING STUD WALLS SHALL BE CONTINUOUSLY BRIDGED AT MID-HEIGHT AND UNSUPPORTED PLYWOOD WALL SHEATHING JOINTS WITH SOLID WOOD BLOCKING, UNLESS NOTED OTHERWISE.
13. NO CUTS, HOLES, OR COPES IN STRUCTURAL WOOD FRAMING SHALL BE PERMITTED WITHOUT PRIOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER AND ARCHITECT.
14. ALL BOLTS, CARRIAGE BOLTS, LAG SCREWS, EXPANSION BOLTS AND EPOXY BOLTS SHALL BE INSTALLED WITH STANDARD CUT WASHERS UNDER THE BOLT HEADS AND NUTS THAT BEAR DIRECTLY ON THE WOOD. ALL NUTS SHALL BE TIGHTENED AT THE TIME OF INSTALLATION AND RE-TIGHTENED IF NECESSARY, DUE TO WOOD SHRINKAGE, PRIOR TO CLOSE-IN OR A THE COMPLETION OF THE PROJECT. BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1. WOOD SCREWS SHALL CONFORM TO B18.6.1. ALL BOLTS SHALL CONFORM TO ASTM A307 GRADE A UNLESS NOTED OTHERWISE. THE MINIMUM STRENGTHS FOR LAG SCREWS AND WOOD SCREWS SHALL BE AS FOLLOWS:

SCREW YIELD STRENGTH		
SCREW DIAMETER	SCREW TYPE	YIELD STRENGTH
#6 (0.138 IN.)	WOOD SCREW	100 KSI
#7 (0.151 IN.)	WOOD SCREW	90 KSI
#8 (0.164 IN.)	WOOD SCREW	90 KSI
#9 (0.177 IN.)	WOOD SCREW	90 KSI
#10 (0.190 IN.)	WOOD SCREW	80 KSI
#12 (0.216 IN.)	WOOD SCREW	80 KSI
#14 (0.246 IN.)	WOOD SCREW	70 KSI
¼ IN.	LAG SCREW	70 KSI
$^{5}/_{16}$ IN.	LAG SCREW	60 KSI
⅜ IN. AND GREATER	LAG SCREW	45 KSI

15. BOLT HOLES SHALL BE CAREFULLY CENTERED AND DRILLED NOT MORE THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTED CONNECTIONS SHALL BE SNUG TIGHT BUT NOT TO THE EXTENT OF CRUSHING WOOD UNDER WASHERS.
16. LAG SCREWS SHALL BE BORED PER NDS 12.1.4
17. ALL PLATES, ANCHORS, NAILS, BOLTS, NUTS, WASHERS, AND OTHER MISCELLANEOUS HARDWARE SHALL BE HOT DIP GALVANIZED IN PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOODS.
18. ALL NAILS FOR STRUCTURAL WORK SHALL BE COMMON WIRE NAILS UNLESS NOTED OR DETAILED OTHERWISE MEETING ASTM F1667. HOLES SHALL BE PRE-DRILLED WHERE NECESSARY TO PREVENT SPLITTING. NAILS SHALL HAVE THE MINIMUM PROPERTIES SPECIFIED IN THE TABLE BELOW.

NAIL YIELD STRENGTH	
NAIL SIZE	YIELD STRENGTH
6d (0.133 IN. x 2 IN.)	100 KSI
8d (0.131 IN. x 2 ½ IN.)	90 KSI
10d (0.148 IN. x 3 IN.)	90 KSI
12d (0.148 IN. x 3 ¼ IN.)	90 KSI
16d (0.162 IN. x 3 ½ IN.)	80 KSI
20d (0.192 IN. x 4 IN.)	80 KSI

19. PROVIDE DOUBLE JOISTS OR SOLID BLOCKING AT 24" O.C. UNDER ALL PARTITIONS AND TO SUPPORT CONCENTRATED LOADS FROM FRAMING ABOVE, UNLESS NOTED OTHERWISE.
20. CUTTING AND NOTCHING OF SAWN LUMBER JOISTS, SAWN LUMBER RAFTERS AND STUDS SHALL BE IN CONFORMANCE WITH THE FOLLOWING CRITERIA:
   A. JOISTS - NOTCHES AT THE ENDS OF JOISTS SHALL NOT EXCEED 1/5TH THE JOIST DEPTH. HOLES BORED IN JOISTS SHALL NOT BE WITHIN 2½ INCHES OF THE TOP OR BOTTOM OF THE JOIST, AND THE DIAMETER OF ANY SUCH HOLE SHALL NOT EXCEED 1/4TH THE DEPTH OF THE JOIST. NOTCHES IN THE TOP OR BOTTOM OF THE JOISTS SHALL NOT EXCEED 1/6TH THE DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE THIRD OF THE SPAN.
   B. RAFTERS - NOTCHING AT THE ENDS OF RAFTERS OR CEILING JOISTS SHALL NOT EXCEED 1/5TH THE DEPTH. NOTCHES IN THE TOP OR BOTTOM OF THE RAFTER OR CEILING JOIST SHALL NOT EXCEED 1/6TH THE DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE 1/3RD OF THE SPAN, EXCEPT THAT A NOTCH NOT EXCEEDING 1/3RD OF THE DEPTH IS PERMITTED IN THE TOP OF THE RAFTER OR CEILING JOIST NOT FURTHER FROM THE FACE OF THE SUPPORT THAN THE DEPTH OF THE MEMBER. HOLES BORED IN RAFTERS OR CEILING JOISTS SHALL NOT BE 2½ INCHES OF THE TOP AND BOTTOM AND THEIR DIAMETER SHALL NOT EXCEED 1/4TH THE DEPTH OF THE MEMBER.
   C. WALL STUDS - A MAXIMUM OF 2½ -INCH DIAMETER NEATLY BORED HOLE MAY BE PLACED IN THE CENTER OF ALL BEARING 2x6 STUDS WITH NO ADDITIONAL REINFORCEMENT REQUIRED.
21. CUTTING AND NOTCHING OF ENGINEERED WOOD PRODUCTS SHALL ADHERE TO THE ALLOWANCES PROVIDED BY THE MANUFACTURER.

WOOD NAILING

REQUIRED COMMON NAIL SIZES		
MARK	SHANK DIA.	LENGTH
8d	0.131"	2 1/2"
10d	0.148"	3"
16d	0.162"	3 1/2"

DESCRIPTION OF BUILDING ELEMENTS	FASTENERS QTY. & TYPE	SPACING & LOCATION
WALLS		
STUD TO TOP PLATE OR BOTTOM PLATE	4-8d COMMON 4-3"x0.131 NAILS	TOENAIL
	2-16d COMMON 3-3"x0.131 NAILS	END NAIL
TOP PLATE LAP SPLICE (INTERIOR NON-SHEARWALLS ONLY)	8-16d COMMON 12-3"x0.131 NAILS	EACH SIDE OF END JOINT, FACE NAIL (MIN 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT)
TOP PLATE LAPS AT CORNERS AND INTERSECTIONS	2-16d COMMON 3-3"x0.131 NAILS	FACE NAIL
TOP PLATE TO TOP PLATE	16d COMMON	16" O.C. FACE NAIL
	3"x0.131 NAILS	12" O.C. FACE NAIL
BOTTOM PLATE TO WOOD FRAMING BELOW (NON-SHEARWALLS ONLY)	16d COMMON	16" O.C. FACE NAIL
	3"x0.131 NAILS	12" O.C. FACE NAIL
CONTINUOUS HEADER TO JAMB STUDS	4-8d COMMON	TOENAIL
HEADER TO KING STUDS	3-10 COMMON	TOENAIL EA. FACE
FLOORS		
JOIST OR TRUSS TO TOP PLATE OR GIRDER BELOW	3-8d COMMON 3-3"x0.131 NAILS	TOENAIL
RIM JOIST, BAN JOIST, OR BLOCKING TO TOP PLATE, SILL OR OTHER FRAMING BELOW	8d COMMON 3"x0.131 NAILS	6" O.C. TOENAIL
JOIST TO BAND JOIST OR RIM JOIST	3-16d COMMON 4-3"0.131 NAILS	END NAIL
MULTIPLE-PLY BEAMS, HEADERS, OR GIRDERS	6 / S3-00	
BRIDGING OR BLOCKING TO JOIST, RAFTER, OR TRUSS	2-8d COMMON 2-3"x0.131 NAILS	EA. END, TOENAIL
ROOFS		
BLOCKING BETWEEN CEILING JOISTS, RAFTERS, OR TRUSSES TO TOP PLATE OR OTHER FRAMING BELOW	3-8d COMMON OR 3-3"x0.131 NAILS	EA. END, TOENAIL
BLOCKING BETWEEN RAFTERS OR TRUSS NOT AT THE WALL TOP PLATE, TO RAFTER OR TRUSS	2-8d COMMON OR 2-3"x0.131 NAILS	EA. END, TOENAIL
	2-16d COMMON OR 3-3"x0.131 NAILS	END NAIL
FLAT BLOCKING TO TRUSS AND WEB FILLER	16d COMMON OR 3"x0.131 NAILS	6" O.C. FACE NAIL
CEILING JOIST TO TOP PLATE	3-8d COMMON OR 3-3"x0.131 NAILS	TOE NAIL
CEILING JOIST NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS (NO THRUST) (SEE IBC 2308.7.3.1, TABLE 2308.7.3.1)	3-16d COMMON OR 4-3"x0.131 NAILS	FACE NAIL
COLLAR TIE TO RAFTER	3-10d COMMON OR 4-3"x0.131 NAILS	FACE NAIL
RAFTER OR ROOF TRUSS TO TOP PLATE (SEE IBC 2308.7.5, TABLE 2308.7.5)	3-10d COMMON OR 4-3"x0.131 NAILS	TOENAIL
ROOF RAFTERS TO RIDGE VALLEY OR HIP RAFTERS; OR ROOF RAFTER TO 2-INCH RIDGE BEAM	2-16d COMMON OR 3-3"x0.131 NAILS	END NAIL
	3-10d COMMON OR 4-3"x0.131 NAILS	TOENAIL

WOOD CONNECTOR TABLE

WOOD CONNECTOR TABLE (SIMPSON STRONG TIE)			
RAFTER / JOIST / BEAM MOUNT HANGER			
MEMBER	BASIC	CONCEALED	SLOPED/SKEWED
2x6	LUS26	LUC26Z	LSSR26Z
2x8	LUS28	LUC26Z	LSSR28Z
2x10	LUS210	LUC210Z	LSSR210Z
2x12	LUS210	LUC210Z	LSSR210Z
(2) 2x8	HU28-2	HUC28-2	--
(2) 2x10	HU210-2	HUC210-2	LSSR210-2Z
(2) 2x12	HU210-2	HUC12-2	LSSR210-2Z
(3) 2x8	HU26-3	HUC26-3	--
(3) 2x10	HU210-3	HUC210-3	--
(3) 2x12	HU210-3	HUC212-3	--
1 3/4"x7 1/4"	HU7	--	--
1 3/4"x9 1/4"	HU9	HUCQ1.81/9-SDS	LSSR1.81Z
1 3/4"x11 1/4"	HU11	HUCQ1.81/11-SDS	LSSR1.81Z
1 3/4"x14"	HU14	HUCQ1.81/11-SDS	--
1 3/4"x16"	HU14	HUCQ1.81/11-SDS	--
(2) 1 3/4"x7 1/4"	HU48	HUC48	--
(2) 1 3/4"x9 1/4"	HU410	HUCQ410-SDS	LSSR410Z
(2) 1 3/4"x11 1/4"	HU412	HUCQ412-SDS	LSSR410Z
(2) 1 3/4"x14"	HU416	HUCQ412-SDS	--
(2) 1 3/4"x16"	HU416	HUCQ412-SDS	--
(3) 1 3/4"x7 1/4"	HU68	HUC68	--
(3) 1 3/4"x9 1/4"	HU610	HUCQ610-SDS	--
(3) 1 3/4"x11 1/4"	HU612	HUCQ612-SDS	--
(3) 1 3/4"x14"	HUC616	HUCQ612-SDS	--
(3) 1 3/4"x16"	HUC616	HUCQ612-SDS	--

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE

GEORGIA REGISTERED No. SE000801 STRUCTURAL ENGINEER MICHAEL CODY THARPE

FOR PERMIT

THARPE ENGINEERING GROUP
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TEG PROJ: 26.013 | DRAWN: GMB | CHECKED: SJP

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GENERAL NOTES

2026.04	
06/17/2026	
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Checked By	SJP

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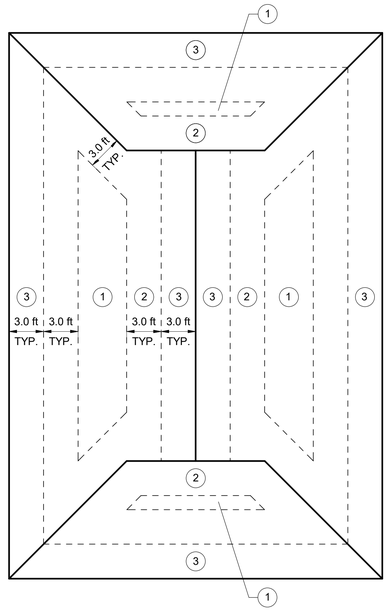

COMPONENTS AND CLADDING DIAGRAM

COMPONENTS AND CLADDING ULTIMATE WIND PRESSURES				
ELEMENT	ZONE	AREA (SQ. FT.)	p_{net} (PSF)	
			POSITIVE	NEGATIVE
ROOF	1	<=10	34.1	-25.7
		50	34.1	-25.7
		100	34.1	-25.7
		>200	34.1	-25.7
	2	<=10	52.5	-39.9
		50	34.1	-25.7
		100	34.1	-25.7
		>200	34.1	-25.7
	3	<=10	52.5	-39.9
		50	34.1	-25.7
		100	34.1	-25.7
		>200	34.1	-25.7

A COMPONENTS & CLADDING DIAGRAM
Not To Scale

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE
1	Response to Permitting Comments	7/10/2026

GEORGIA REGISTERED No. PE000801 STRUCTURAL 06/17/2026 ENGINEER MICHAEL CODY THARPE

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C&C DIAGRAM

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STRUCTURAL SPECIAL INSPECTIONS

1.	Special structural tests and inspections shall be performed on this project in accordance with the requirements of Chapter 17 of the International Building Code (IBC). The following documents have been prepared for this project as a part of these construction documents:	
	A.	Statement of Special Inspections
	B.	Schedule of Special Inspections
	C.	Statement of Special Inspections Requirements for Wind Resistance
	D.	Statement of Special Inspections Requirements for Seismic Resistance
2.	Special structural tests and inspections shall be performed by an agency selected by the contractor and approved by the design professional in responsible charge. The agency shall meet all of the requirements for approval indicated in IBC Section 1703.1. Special inspectors shall be qualified persons who shall demonstrate competence to the satisfaction of the building official for inspection of the particular type of construction or operation requiring special inspection.	
3.	The contractor shall coordinate the inspection services in accordance with the progress of the work. The contractor shall provide sufficient notice to the inspector to allow proper scheduling of personnel.	
4.	All reports and shop certification of special inspections to be performed on the premises of a fabricator's shop shall be submitted to the contractor. The contractor shall be responsible for distributing these reports to the special inspector, the architect, and the engineer of record in a timely manner.	
5.	The costs of the special inspector's services shall be paid for by the owner. Special inspections reports and a final report in accordance with IBC Section 1704.2.4 shall be submitted to the building official prior to the time that phase of the work is approved for occupancy.	
6.	Reports shall indicate that the work was performed and constructed in accordance with the contract documents. Work not in conformance shall be identified in the report and shall be brought to the immediate attention of the contractor.	
7.	A final report of inspections documenting required special inspections, including any discrepancies noted in the inspections shall be submitted to the building official, the architect, and the engineer of record prior to completion of the structural systems but at a frequency not to exceed 60 days.	

STATEMENT OF SPECIAL INSPECTIONS

Project	Savannah Quarters Pavilion
Location	8 Palladian Way, Pooler, GA
Permit Application	
Applicant's Address	
Architect of Record	
Structural Engineer of Record	M. Cody Tharpe, P.E.
Mechanical Engineer of Record	
Electrical Engineer of Record	
Registered Design Professional in Charge	

This Statement of Special Inspections is submitted in accordance with Section 1704.3 of the 2018 International Building Code. It includes a Schedule of Special Inspection Services applicable to the above-referenced Project as well as the identity of the individuals, agencies, or firms intended to be retained for conducting these inspections. If applicable, it includes Special Inspections for Seismic Resistance and/or Special Inspections for Wind Resistance.

Are Special Inspections for Seismic Resistance included in the Statement of Special Inspections?	YES
Are Special Inspections for Wind Resistance included in the Statement of Special Inspections?	YES

The Special Inspector(s) shall keep records of all inspections and shall furnish interim inspection reports to the Building Official and to the Registered Design Professional in Responsible Charge at a frequency agreed upon by the Design Professional and the Building Official prior to the start of work. Discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge prior to completion of that phase of work. A Final Report of Special Inspections documenting required special inspections and corrections of any discrepancies noted in the inspections shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge at the conclusion of the project.

Frequency of interim report submittals to the Registered Design Professional in Responsible Charge:

_X_ Weekly	___ Bi-Weekly	___ Monthly	Other; specify:

The Special Inspection program does not relieve the Contractor of the responsibility to comply with the Contract Documents. Jobsite safety and means and methods of construction are solely the responsibility of the Contractor.

Statement of Special Inspections Prepared by:

Type or print name:	M. Cody Tharpe, P.E.		
Signature	M Cody Tharpe	Date	06/17/2026

Building Official's Acceptance:

Signature		Date	
Permit Number			

Frequency of interim report submittals to the Building Official:

___ Monthly	___ Bi-Monthly	___ Upon Completion	Other; specify:

Preparer's Seal:

GEORGIA REGISTERED No. SE000801 STRUCTURAL ENGINEER MICHAEL CODY THARPE

SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE

See the Schedule of Special Inspections for inspection and testing requirements

Seismic Design Category	**C**
Special Inspections for Seismic Resistance Required	NO

Description of seismic force-resisting system subject to special inspection and testing for seismic resistance:
(Where required per IBC Sections 1705.13.1, 1705.13.2, and 1705.13.3) (Special inspections for seismic resistance of structural steel, where required, shall be in accordance with AISC 341)

N/A

Description of designated seismic systems subject to special inspection and testing for seismic resistance:
(Required for architectural, electrical and mechanical systems and their components that require design in accordance with Chapter 13 of ASCE 7, have a component importance factor, Ip, greater than one and are in Seismic Design Categories C, D, E or F.)

N/A

Description of additional seismic systems and components requiring special inspections:
(Required for systems noted in IBC Section 1705.13.5, 1705.13.6, 1705.13.7, 1705.13.8, and 1705.13.9.)

N/A

Description of additional seismic systems and components requiring testing:
(Where required per IBC Section 1705.14)

N/A

Statement of Responsibility:
Each contractor responsible for the construction or fabrication of a system or component described above must submit a Statement of Responsibility.

SPECIAL INSPECTIONS FOR WIND RESISTANCE

See the Schedule of Special Inspections for inspection and testing requirements

Allowable Stress Design Wind Speed, V_{asd} (m.p.h.)	C
Wind Exposure Category	102
Special Inspections for Wind Resistance Required: (Required in wind exposure Category B, where the allowable stress design wind speed, V_{asd}, is 120 miles per hour or greater. Required in wind exposure Category C or D, where the allowable stress design wind speed, V_{asd}, is 110 miles per hour or greater).	NO

Description of structural wood and cold-formed steel light frame construction main windforce-resisting system subject to special inspections for wind resistance:
(Required for systems noted in IBC Section 1705.12.1 and 1705.12.2).

N/A

Description of windforce-resisting components subject to special inspections for wind resistance:
(Required for systems and components noted in IBC Section 1705.12.3)

N/A

Statement of Responsibility:
Each contractor responsible for the construction or fabrication of a system or component described above must submit a Statement of Responsibility.

FINAL REPORT OF SPECIAL INSPECTIONS

Project	Savannah Quarters Pavilion
Location	8 Palladian Way, Pooler, GA
Permit Application	
Applicant's Address	
Architect of Record	
Structural Engineer of Record	M. Cody Tharpe, P.E.
Mechanical Engineer of Record	
Electrical Engineer of Record	
Registered Design Professional in Charge	

To the best of my information, knowledge, and belief, which are based upon observations or diligent supervision of our inspection services for the above-referenced Project, I hereby state that the special inspections or testing required for this Project and designated for this Agent in the Schedule of Special Inspection Services, have been completed in accordance with the Contract Documents.

The Special Inspection program does not relieve the Contractor of the responsibility to comply with the Contract Documents. Jobsite safety and means and methods of construction are solely the responsibility of the Contractor.

Interim reports submitted prior to this final report and numbered to form a basis for and are to be considered an integral part of this final report. The following discrepancies that were outstanding since the last interim report dated have been corrected:

Prepared by:

Special Inspection Agent/Firm:			
Type or print name:			
Signature:		Date:	

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE

GEORGIA REGISTERED No. SE000801 STRUCTURAL ENGINEER MICHAEL CODY THARPE

FOR PERMIT

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SPECIAL INSPECTIONS FORMS

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	06/17/2026
Drawn By	GMB
Checked By	SJP

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SCHEDULE OF SPECIAL INSPECTION SERVICES

Project	

The following sheets comprise the required schedule of special inspections for this project. The construction divisions which require special inspections for this project are as follows.

Concrete Construction	
Wood Construction	
Soils	

	MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT		
			Y/N	EXTENT	AGENT
1705.3 Concrete Construction					
1.	Inspect reinforcement, including prestressing tendons, and verify placement.	Shop (3) and field inspection	Y	Periodic	1
2.	Reinforcing Bar Welding:				
A.	Verification of weldability of bars other than ASTM A706.	Shop (3) and field inspection	N	Periodic	1
B.	Inspect welding of reinforcement for special moment frames, boundary elements of special structural walls and coupling beams.	Shop (3) and field inspection	N	Continuous	1
C.	Inspect welded reinforcement splices.	Shop (3) and field inspection	N	Continuous	1
D.	Inspect welding of primary tension reinforcement in corbels.	Shop (3) and field inspection	N	Continuous	1
E.	Inspection of single-pass fillet welds 5/16 or less in size.	Shop (3) and field inspection	N	Periodic	1
F.	Inspection of all other welds.	Shop (3) and field inspection	N	Continuous	1
3.	Inspection of anchors cast in concrete.	Shop (3) and field inspection	Y	Periodic	1
4.	Inspection of anchors post-installed in hardened concrete members per research reports, or, if no specific requirements are provided, requirements shall be provided by the registered design professional and approved by the building official, including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	N	Periodic or as required by the research report issued by an approved source	1
A.	Adhesive anchors installed in horizontally or upwardly-inclined orientation that resist sustained tension loads.	Field inspection	N	Continuous	1
B.	Mechanical and adhesive anchors not defined in 4a.	Field inspection	N	Periodic	1
5.	Verify use of approved design mix	Shop (3) and field inspection	Y	Periodic	1
6.	Prior to placement:				
A.	Fabricate specimens for strength tests, fresh concrete sampling, perform slump or slump flow, and air content density tests, and determine temperature of concrete.	Shop (3) and field inspection	Y	Continuous	1
B.	Verify that concrete specimens for strength tests are maintained in the required initial curing and laboratory curing environment, and that the maximum and minimum temperatures during the initial curing period are reported.	Shop (3) and field inspection	Y	Continuous	1
7.	Inspection of concrete and shotcrete placement for proper application techniques	Shop (3) and field inspection	Y	Continuous	1
8.	Verify maintenance of specified curing temperature and techniques	Shop (3) and field inspection	Y	Periodic	1
9.	Inspection of prestressed concrete:				
A.	Application of prestressing force	Shop (3) and field inspection	N	Continuous	1
B.	Grouting of bonded prestressing tendons	Shop (3) and field inspection	N	Continuous	1
10.	Inspect erection of precast concrete members	Field Inspection	N	Periodic	1
11.	For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to Seismic Design Category C, D, E, or F, inspect such connections and reinforcement in the field for:				
A.	Installation of the embedded parts.	Field Inspection	N	Continuous	1
B.	Completion of the continuity of reinforcement across joints.	Field Inspection	N	Continuous	1
C.	Completion of connections in the field.	Field Inspection	N	Continuous	1
12.	Inspect installation tolerances of precast concrete diaphragm connections for compliance with ACI 550.5.	Field Inspection	N	Periodic	1
13.	Verify in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports	N	Periodic	1
14.	Inspection of formwork for shape, lines, location and dimensions	Field inspection	N	Periodic	1
15.	Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	N	Periodic	1
1705.5 Wood Construction					
1.	For prefabricated wood structural elements, inspection of the fabrication process and assemblies in accordance with Section 1704.2.5.	In-plant review (3)	Y	Periodic	1
2.	For high-load diaphragms, verify grade and thickness of structural panel sheathing agree with approved building plans.	Field inspection	N	Periodic	1
3.	For high-load diaphragms, verify nominal size of framing members at adjoining panel edges, nail or staple diameter and length, number of fastener lines, and that spacing between fasteners in each line and at edge margins agree with approved building plans	Field inspection	N	Periodic	1
4.	Metal-Plate-Connected Wood Trusses:				
A.	Verification that permanent individual truss member restraint/bracing has been installed in accordance with the approved truss submittal package when the truss height is greater than or equal to 60".	Field inspection	N	Periodic	1
B.	For trusses spanning 60 feet or greater: verify temporary and permanent restraint/bracing are installed in accordance with the approved truss submittal package	Field inspection	N	Periodic	1
1705.6 Soils					
1.	Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	Field inspection	Y	Periodic	1
2.	Verify excavations are extended to proper depth and have reached proper material.	Field inspection	Y	Periodic	1
3.	Perform classification and testing of compacted fill materials.	Field inspection	Y	Periodic	1
4.	During fill placement, verify use of proper materials and procedures in accordance with the provisions of the approved geotechnical report. Verify densities and lift thicknesses during placement and compaction of compacted fill.	Field inspection	Y	Continuous	1
5.	Prior to placement of controlled fill, inspect subgrade and verify that site has been prepared properly	Field inspection	Y	Periodic	1

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE

GEORGIA REGISTERED No. SE000801 STRUCTURAL 06/17/2026 ENGINEER MICHAEL CODY THARPE

FOR PERMIT

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SPECIAL INSPECTIONS SCHEDULES

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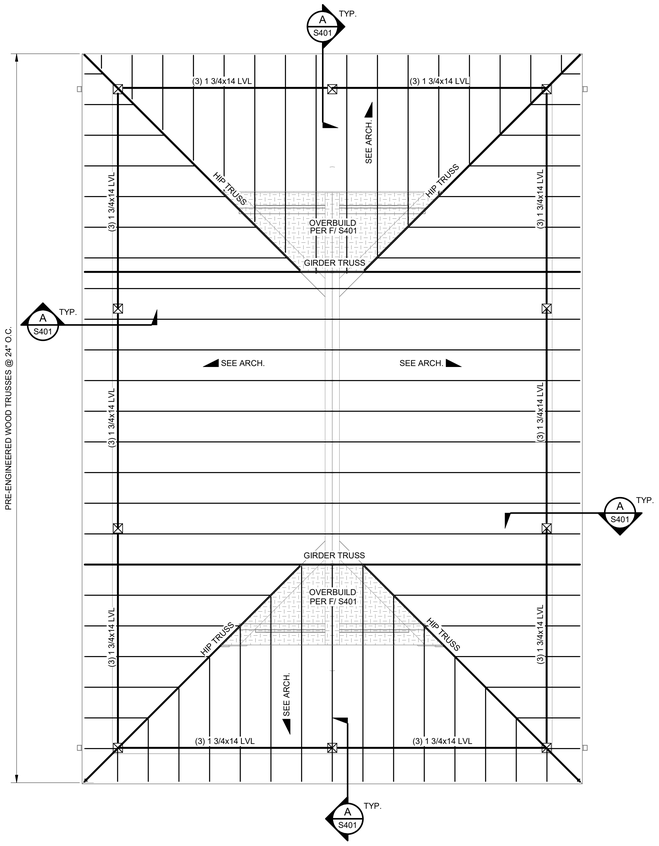

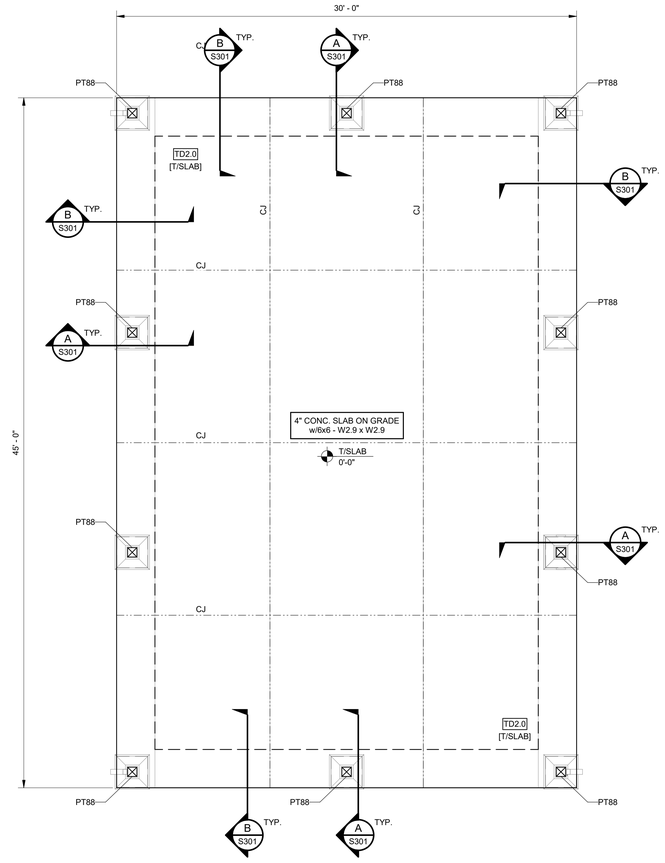

ROOF FRAMING PLAN NOTES:

1. BASE ELEV. (0'-0") IS FOR REFERENCE ONLY. COORD. ELEV. w/ARCH. & CIVIL DWGS.
2. ROOF TYPICAL TBE = 12'-0" FROM BASE ELEVATION/T/SLAB
3. ALL WOOD FRAMING EXPOSED TO THE ELEMENTS SHALL BE PRESSURE TREATED.
4. ROOF TRUSSES, TRUSS GIRDERS, AND TRUSS HEADERS SHALL BE DESIGNED BY OTHERS UNLESS NOTED OTHERWISE.
5. TRUSS MANUFACTURER SHALL SPECIFY ALL TRUSS TO TRUSS AND TRUSS TO BEAM CONNECTIONS.
6. TRUSS MANUFACTURER SHALL VERIFY/COORDINATE CEILING MOUNTED UNIT WEIGHTS AND LOCATIONS WITH MECH DWGS.
7. ALL EXTERIOR WALL SHEATHING SHALL BE:
   A. 15/32" APA-RATED 24/16 MIN. WOOD STRUCTURAL PANELS WITH EXTERIOR GRADE GLUE & 8D COMMON NAILS (0.131"ØX2.5" LONG) AS FOLLOWS.
      a. SUPPORTED EDGES & DIAPHRAGM BOUNDARIES: 4" O.C.
      b. FIELD & INTERMEDIATE SUPPORTS: 12" O.C.
8. PANEL EDGES BACKED WITH 2-INCH NOMINAL OR WIDER FRAMING. INSTALL PANELS EITHER HORIZONTALLY OR VERTICALLY. SPACE FASTENERS MINIMUM 12 INCHES ON CENTER ALONG INTERMEDIATE FRAMING MEMBERS.
9. FRAMING AT ADJOINING PANEL EDGES SHALL BE 3 INCHES NOMINAL OR WIDER, AND NAILS SHALL BE STAGGERED WHERE NAILS ARE SPACED AT 3 INCHES ON CENTER.
10. REFERENCE TYPICAL DETAILS AND SECTIONS ON SHEET S401.

PLAN NORTH

A 1/4" = 1'-0" ROOF FRAMING PLAN

FOUNDATION SCHEDULE

FTG. ID	DIMENSIONS (W'xL'xD')	BOTTOM REINF. LONG.	BOTTOM REINF. TRANS.	TOP REINF. LONG.	TOP REINF. TRANS.	T/FTG. EL.
TD2.0	2'-0"xCONT.x16"D	(3) #5	#5 @10" O.C.	(3) #5	#5 @10" O.C.	T/SLAB

REFER TO THIS SCHEDULE FOR ALL INFORMATION SHOWN ON FOUNDATION SECTIONS OR DETAILS NOTED AS "SEE PLAN" OR "SEE SCHEDULE"

FOUNDATION PLAN NOTES:

1. BASE ELEV. (0'-0") IS FOR REFERENCE ONLY. COORD. ELEV. w/ARCH. & CIVIL DWGS.
2. ALL WOOD FRAMING EXPOSED TO THE ELEMENTS SHALL BE PRESSURE TREATED.
3. SLOPE SLAB AWAY FROM BUILDING WHEN REQUIRED IN THE ARCHITECTURAL DRAWINGS.
4. SUBMIT PROPOSED SOG CONSTRUCTION OR CONTRACTION JOINT LOCATIONS. FOR SCHEMATIC CJ PLANS AS SHOWN ON PLAN.
5. GC SHALL COORDINATE PLUMBING AND UTILITIES LOCATIONS WITH FOUNDATIONS AND FOUNDATION ELEVATIONS AS NEEDED. FORWARD ANY REQUESTED MODIFICATIONS TO THE FOUNDATION ELEVATIONS FOR REVIEW AND APPROVAL BY THE EOR.
6. SEE ARCHITECTURAL DRAWINGS FOR SLAB SLOPING REQUIREMENTS (MAINTAIN MINIMUM THICKNESS NOTED), DEPRESSED AND RAISED SLAB AREAS (MAINTAIN MINIMUM THICKNESS NOTED), AND ALL DIMENSIONS NOT SHOWN IN STRUCTURAL DRAWINGS.
7. GC TO VERIFY ALL DIMENSIONS SHOWN IN THE STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCIES OR DIMENSIONS NOT SHOWN ON ARCHITECTURAL DRAWINGS FOR CLARIFICATION.
8. REFERENCE TYPICAL DETAILS AND SECTIONS ON SHEET S301.

PLAN NORTH

B 1/4" = 1'-0" FOUNDATION PLAN

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE
1	Response to Permitting Comments	7/10/2026

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PLAN VIEWS

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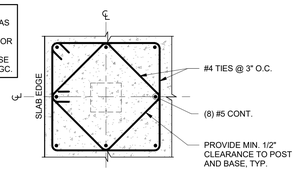
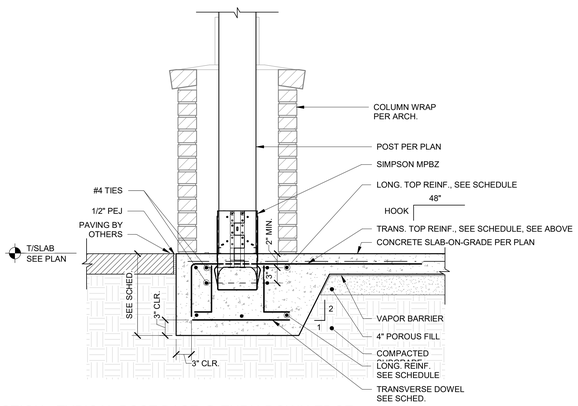
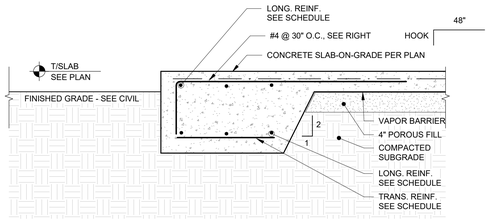
NOTE:
WHERE ANCHORS ARE SPECIFICALLY DETAILED AS CIP, ANCHORS MUST BE EMBEDDED INTO CONCRETE. POST-INSTALLED ANCHORS (EPOXY OR MECHANICAL ANCHORS) ARE NOT SUFFICIENT. PLACING CONCRETE PRIOR TO EMBEDDING THESE CIP ANCHORS WILL RESULT IN REWORK BY THE GC.



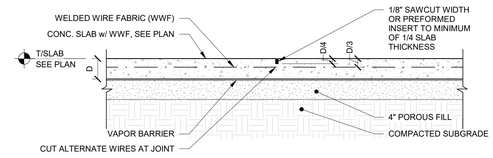
PLAN DETAIL



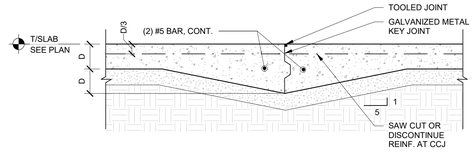
A 1" = 1'-0" SECTION - TYPICAL POST BASE @ TURN-DOWN EDGE



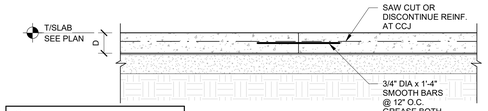
B 1" = 1'-0" SECTION - TYPICAL EXTERIOR SLAB TURN-DOWN EDGE



CONTROL JOINT (CJ)



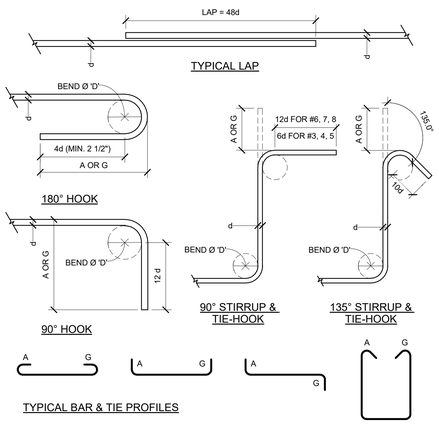
CONSTRUCTION JOINT (CCJ)



NOTE:
STANDARD OR ALTERNATE CCJ MAY BE USED AT G.C.'S DISCRETION.

ALTERNATE CONSTRUCTION JOINT (CCJ)

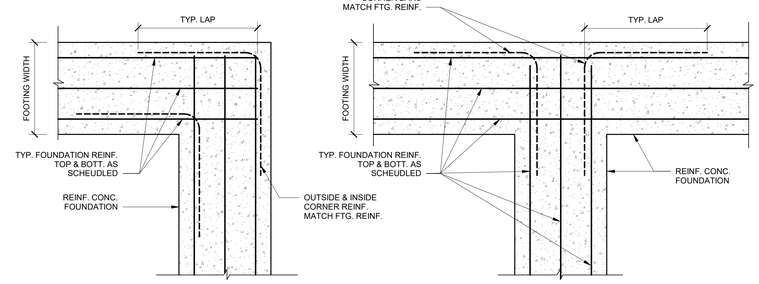
C 1" = 1'-0" TYPICAL SLAB-ON-GRADE DETAILS



TYPICAL LAP & HOOK SCHEDULE							
BAR SIZE	TYP. LAP (IN.)	STANDARD HOOK			STIRRUP & TIE HOOK		
		D (IN.)	180° HOOK A OR G (IN.)	90° HOOK A OR G (IN.)	D (IN.)	90° HOOK A OR G (IN.)	135° HOOK A OR G (IN.)
#3	18	2 1/4	5	6	1 1/2	4	4
#4	24	3	6	8	2	4 1/2	4 1/2
#5	30	3 3/4	7	10	2 1/2	6	5 1/2
#6	36	4 1/2	8	1-0	4 1/2	1-0	8
#7	42	5 1/4[a]	10	1-2	5 1/4[a]	1-2	9
#8	48	6[a]	11	1-4	6[a]	1-4	10 1/2
#9	54	9 1/2	1-3	1-7			
#10	62	10 3/4	1-5	1-10			
#11	68	12	1-7	2-0			
#14	82	18 1/4	2-3	2-7			
#18	108	24	3-0	3-5			

NOTES:
1. ASTM A767 REQUIRES THAT BARS BENT COLD PRIOR TO HOT DIP GALVANIZING MUST BE FABRICATED TO A MINIMUM BEND DIAMETER EQUAL TO 7 IN. FOR #7 BAR AND 8 IN. FOR #8 BAR.
2. UNITS = IN. OR FT.-IN.

D 1" = 1'-0" TYPICAL CAST-IN-PLACE REINFORCING LAP AND HOOK LENGTHS

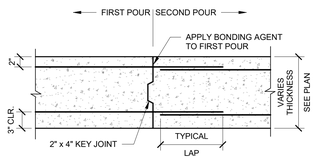


PLAN - CORNER DETAIL

PLAN - INTERSECTION DETAIL

NOTE:
CORNER BARS OCCUR AT BOTH TOP AND BOTTOM WHERE BOTH TOP AND BOTTOM BARS OCCUR IN THE FOUNDATION.

E 1" = 1'-0" TYPICAL REINFORCEMENT @ CONTINUOUS FOOTING INTERSECTIONS



NOTE:
SEE FOUNDATION SCHEDULE ON FOUNDATION PLAN FOR SIZE AND SPACING OF CONT. REINFORCING.

F 1" = 1'-0" TYPICAL CONSTRUCTION JOINT FOR CONTINUOUS FOOTING

SAVANNAH QUARTERS PAVILION
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FOUNDATION SECTIONS & TYPICAL DETAILS

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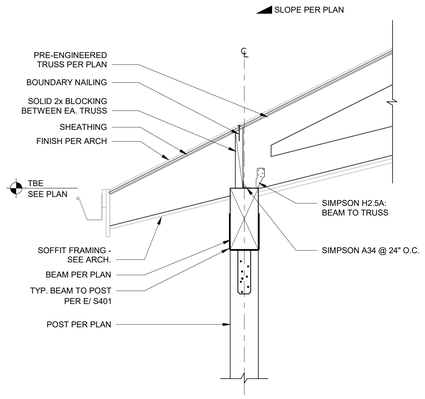
NOTE:
ROOFING, INSULATION, AND FLASHING SHOWN FOR REFERENCE ONLY. SEE ARCH. DWGS.



A 1" = 1'-0" SECTION - VAULTED TRUSS BEARING @ EXTERIOR BEAM

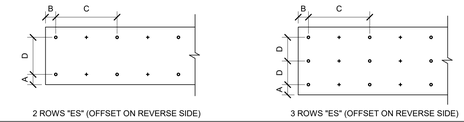
2x & LVL MEMBERS

MINIMUM FASTENING REQUIREMENTS FOR TOP AND SIDE LOADED MEMBERS

	MEMBER DEPTH	2 PLY	3 PLY	4 PLY	5 PLY
10d (0.128"x3") NAILS	7 1/4" ≤ d < 14"	3 ROWS @ 12" O.C.	3 ROWS @ 12" O.C. (ES)	--	--
	d ≥ 14"	4 ROWS @ 12" O.C.	4 ROWS @ 12" O.C. (ES)	--	--
16d (0.162"x3 1/2") NAILS	7 1/4" ≤ d < 14"	--	2 ROWS @ 12" O.C. (ES)	--	--
	d ≥ 14"	--	3 ROWS @ 12" O.C. (ES)	--	--
1/2" THROUGH BOLTS	d ≥ 7 1/4"	2 ROWS @ 24" O.C.	2 ROWS @ 24" O.C.	2 ROWS @ 24" O.C.	2 ROWS @ 24" O.C.
SDS 1/4"x 3 1/2" SCREWS	d ≥ 7 1/4"	--	2 ROWS @ 24" O.C. (ES)	--	--
SDS 1/4"x 6" SCREWS	d ≥ 7 1/4"	--	--	2 ROWS @ 24" O.C. (ES)	2 ROWS @ 24" O.C. (ES)

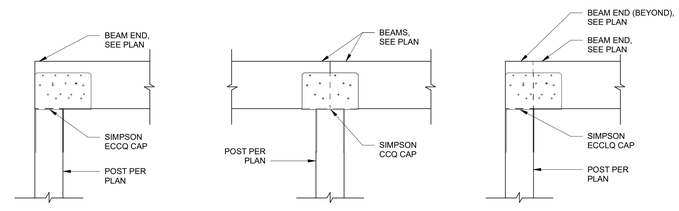
FASTENER CLEARANCES FOR MULTI-PLY MEMBERS

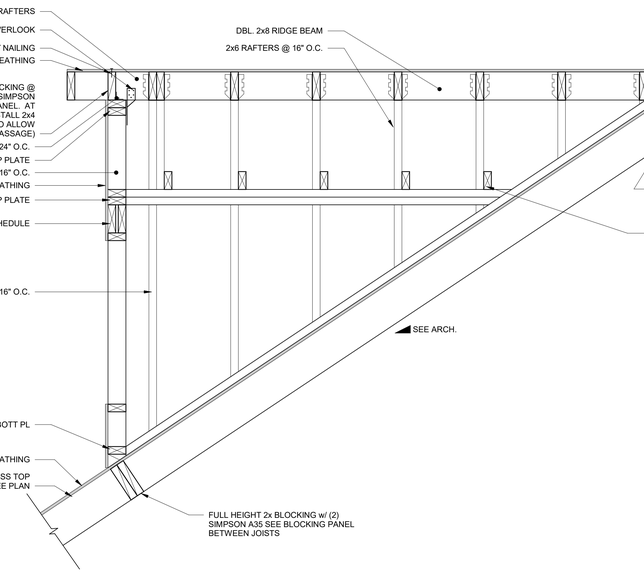
FASTENER	A	B		C		D
	MIN.	MIN.	MAX.	MIN.	MAX.	MIN.
10d & 16d NAILS	2"	2"	6"	4"	12"	3"
BOLTS AND SCREWS	2"	4"	12"	4"	24"	3"



NOTES:
1. ALL FASTENERS MUST HAVE THE LENGTH FULLY EMBEDDED, BUT MUST NOT BE OVERDRIVEN, COUNTERSUNK, OR OVER-TIGHTENED.
2. BOLT HOLES ARE TO BE 1/16" MAX. DIAMETER LARGER THAN THE BOLTS. BOLTS MUST MEET OR EXCEED ASTM A 307. EVERY BOLT MUST EXTEND THROUGH THE FULL THICKNESS OF THE MEMBER. USE WASHERS NOT LESS THAN A STANDARD CUT WASHER UNDER THE HEAD AND NUT MEETING ANSI B 18.22.1.
3. 4-PLY MEMBERS SHOULD ONLY BE SIDE LOADED WHEN LOADS ARE APPLIED TO BOTH SIDES, WHEN THE LESSER SIDE LOAD PLF IS AT LEAST 25% OF THE OPPOSITE SIDE, OR WHEN THE BEAM IS OTHERWISE RESTRAINED TO MINIMIZE ROTATION.

C 1" = 1'-0" TYPICAL FASTENING REQUIREMENTS FOR MULTI-PLY WOOD MEMBERS





D 1" = 1'-0" TYPICAL ROOF SHEATHING

E 1" = 1'-0" TYPICAL WOOD POST TO WOOD BEAM

F 1" = 1'-0" TYPICAL DORMER OVERBUILD

SAVANNAH QUARTERS PAVILION
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FRAMING SECTIONS & TYPICAL DETAILS

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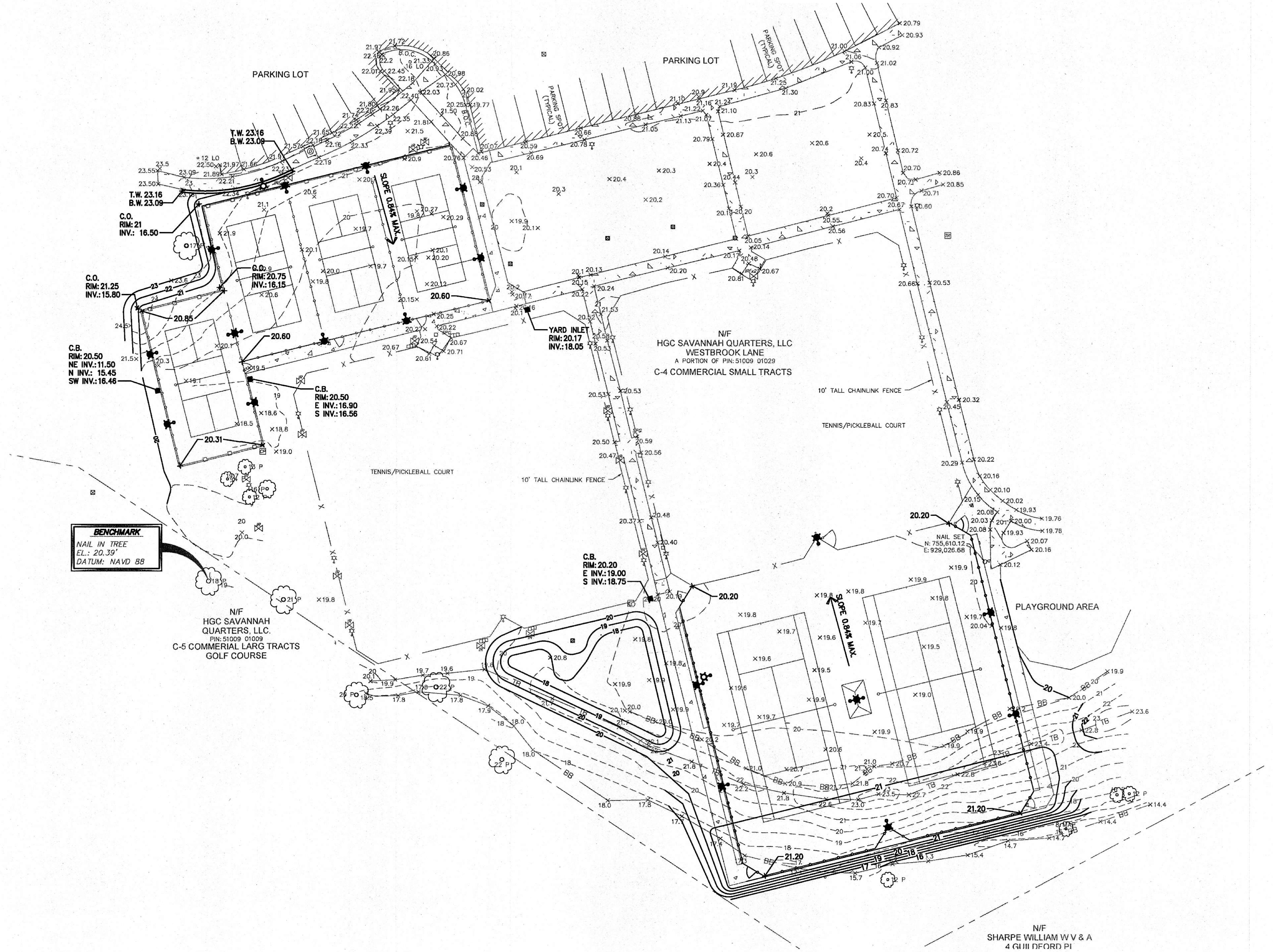

A1 EXISTING CONDITIONS SITE PLAN
1/16" = 1'-0"

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE
1	Response to Permitting Comments	7/10/2026

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STATE OF GEORGIA
STEVEN G. STOWERS
REGISTERED ARCHITECT
6/17/26

FOR CONSTRUCTION

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EXISTING CONDITIONS SITE PLAN

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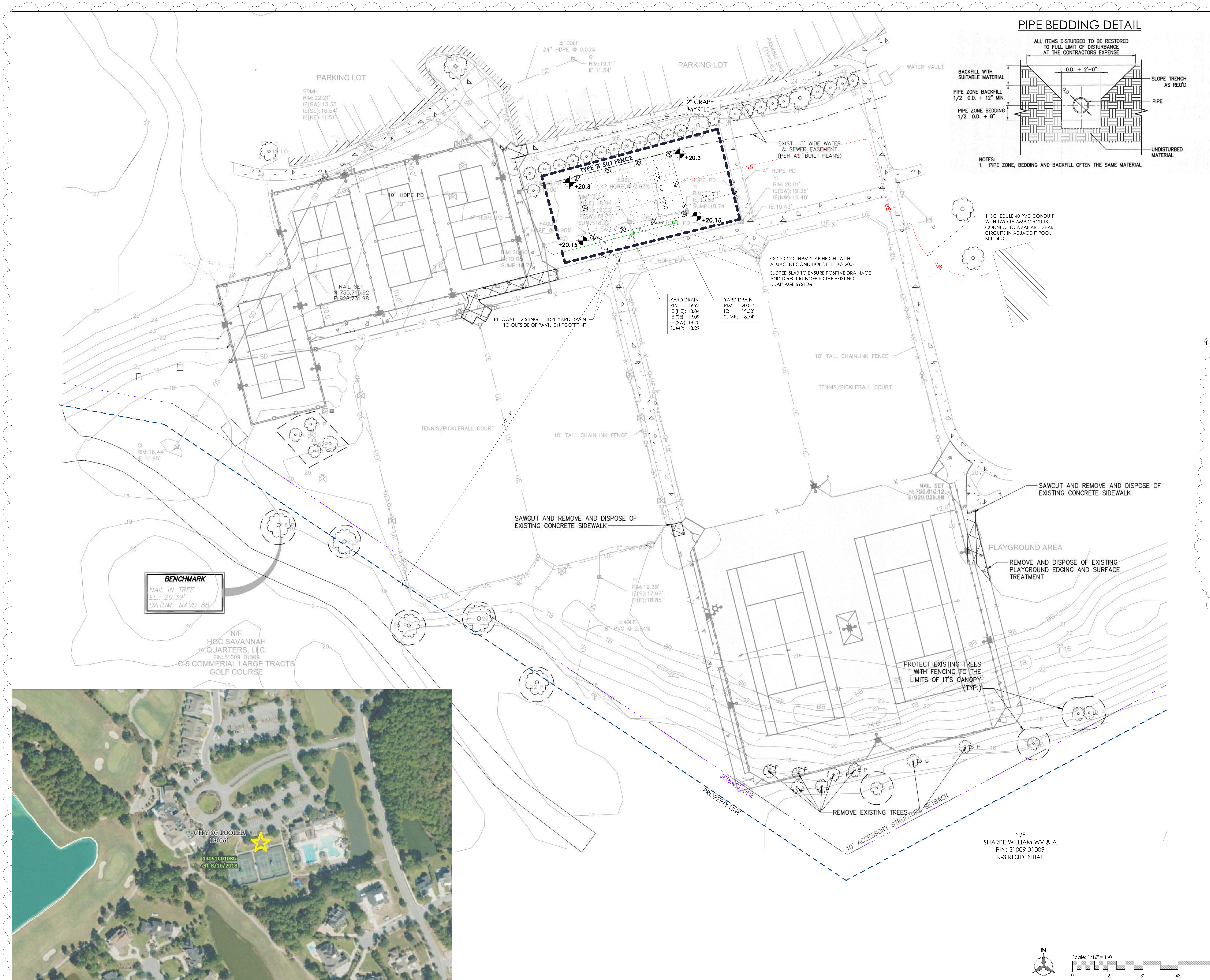

A1 SITE PLAN
1/16" = 1'-0"

PIPE BEDDING DETAIL

ALL ITEMS DISTURBED TO BE RESTORED TO FULL LIMIT OF DISTURBANCE AT THE CONTRACTORS EXPENSE
BACKFILL WITH SUITABLE MATERIAL
PIPE ZONE BACKFILL 1/2 O.D. + 12" MIN.
PIPE ZONE BEDDING 1/2 O.D. + 8"
O.D. + 2'-0"
SLOPE TRENCH AS REQ'D
PIPE
UNDISTURBED MATERIAL

NOTES:
1. PIPE ZONE, BEDDING AND BACKFILL OFTEN THE SAME MATERIAL

SAVANNAH QUARTERS PAVILION
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STATE OF GEORGIA REGISTERED ARCHITECT STEVEN G. STOWERS 6/17/26

FOR CONSTRUCTION

ACREAGE SUMMARY

SITE AREA:	13.03 AC±
ON-SITE DISTURBED AREA:	1.60 AC±
OFF-SITE DISTURBED AREA:	0.00 AC±
TOTAL DISTURBED AREA:	1.60 AC±
IMPERVIOUS AREA AT COMPLETION:	0.69 AC±
NEW IMPERVIOUS AREA = .03 AC±	
PERVIOUS AREA AT COMPLETION:	0.94 AC±

SITE DATA

PARCEL IDENTIFICATION: 51009 01029
OWNER: SAVANNAH QUARTERS COUNTRY CLUB
CURRENT ZONING: C-4

PROPOSED DISTURBED AREA: 1.56 ACRES
15% IMPERVIOUS PICKLEBALL COURTS
23% PERVIOUS PAVER, TENNIS COURT
2% COVERED PAVILION (ADDS 1,350 SF IMPERVIOUS)

BUILDING SETBACKS: 10' ACCESSORY STRUCTURE SETBACK

BUILDING MAXIMUM HEIGHT: 20'-1"

FLOODPLAIN INFORMATION: ZONE X

STATE WATERS: NO STATE WATERS OR WRESTED VEGETATION PRESENT ON SITE

WETLANDS: NO WETLANDS PRESENT ON SITE

SIGNIFICANT TREES & TREE PROTECTION: NO SIGNIFICANT TREES ARE PRESENT IN THE GENERAL AREA OF THE PAVILION THAT WILL REQUIRE CRITICAL ROOT ZONE PROTECTION. ONE LARGE CRAPE MYRTLE IS IN CLOSE PROXIMITY, BUT THE DISTURBED AREA WILL BE OUT OF THE TREE'S CRITICAL ROOT ZONE.

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SITE PLAN

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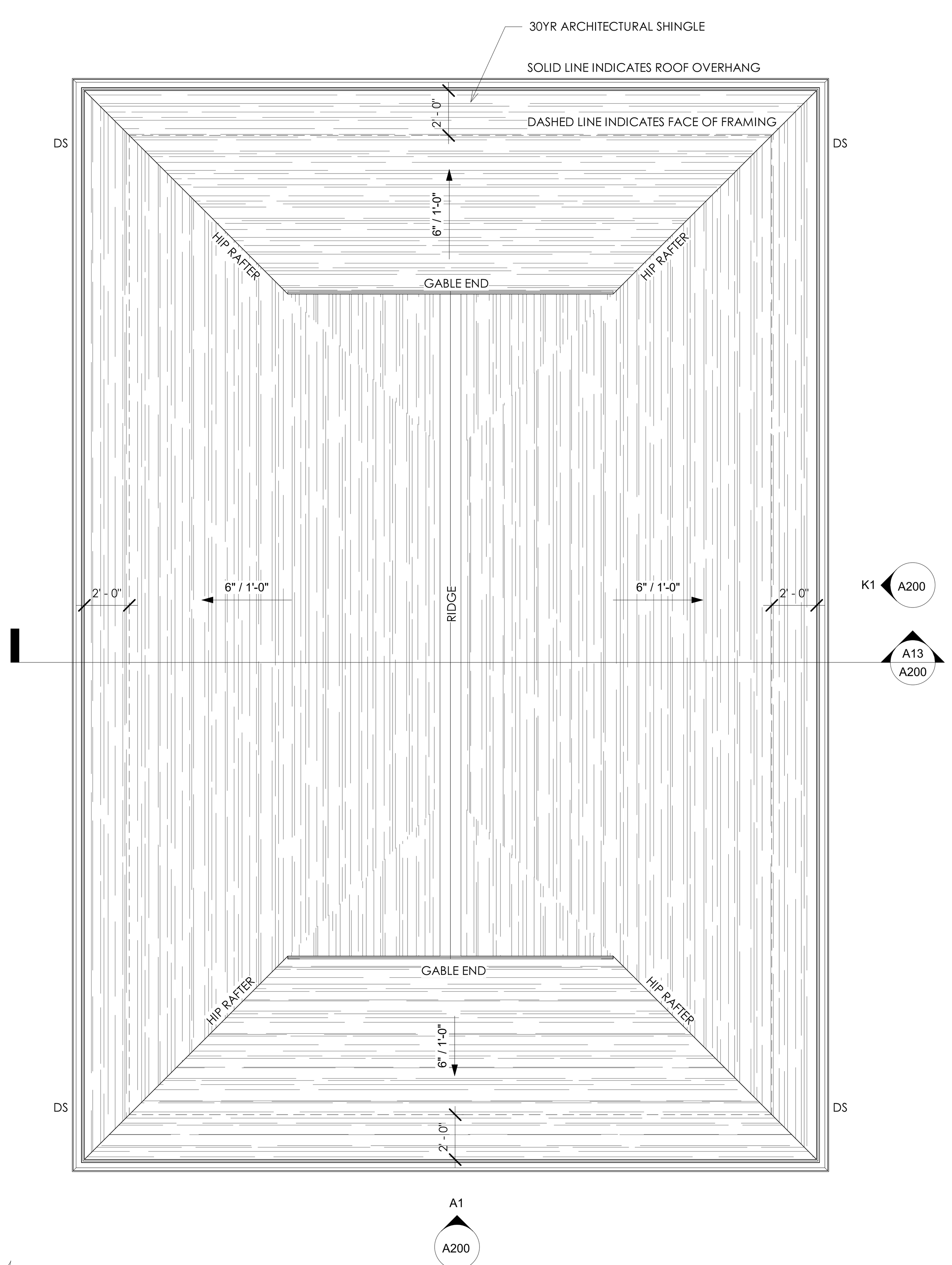
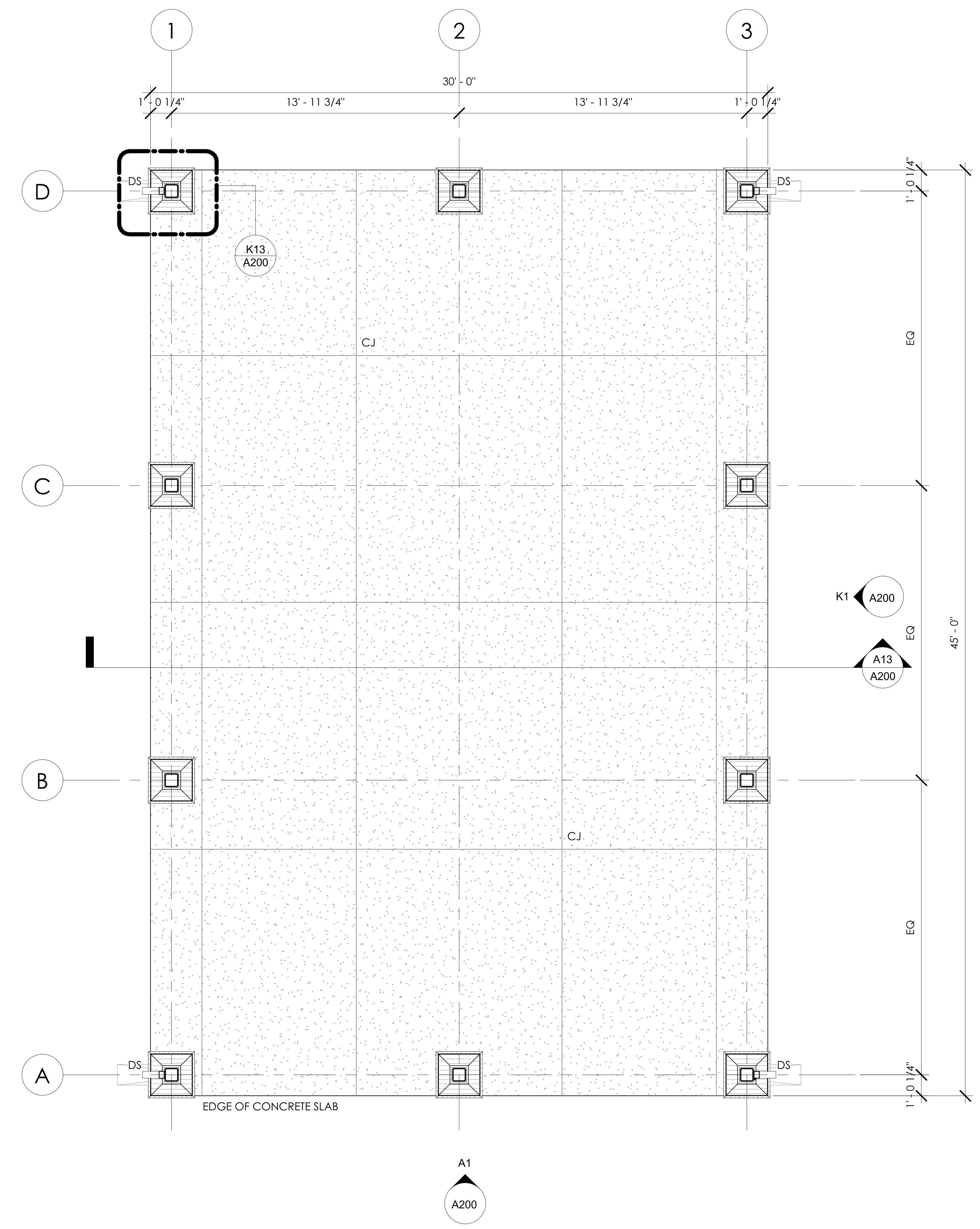


A1 **ROOF PLAN**
1/4" = 1'-0"



A12 **FLOOR PLAN**
1/4" = 1'-0"

Scale: 1/4" = 1'-0"
0 4' 8' 12' 16'

SAVANNAH QUARTERS PAVILION
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STATE OF GEORGIA
STEVEN G. STOWERS
REGISTERED ARCHITECT

FOR CONSTRUCTION

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FLOOR AND ROOF PLANS

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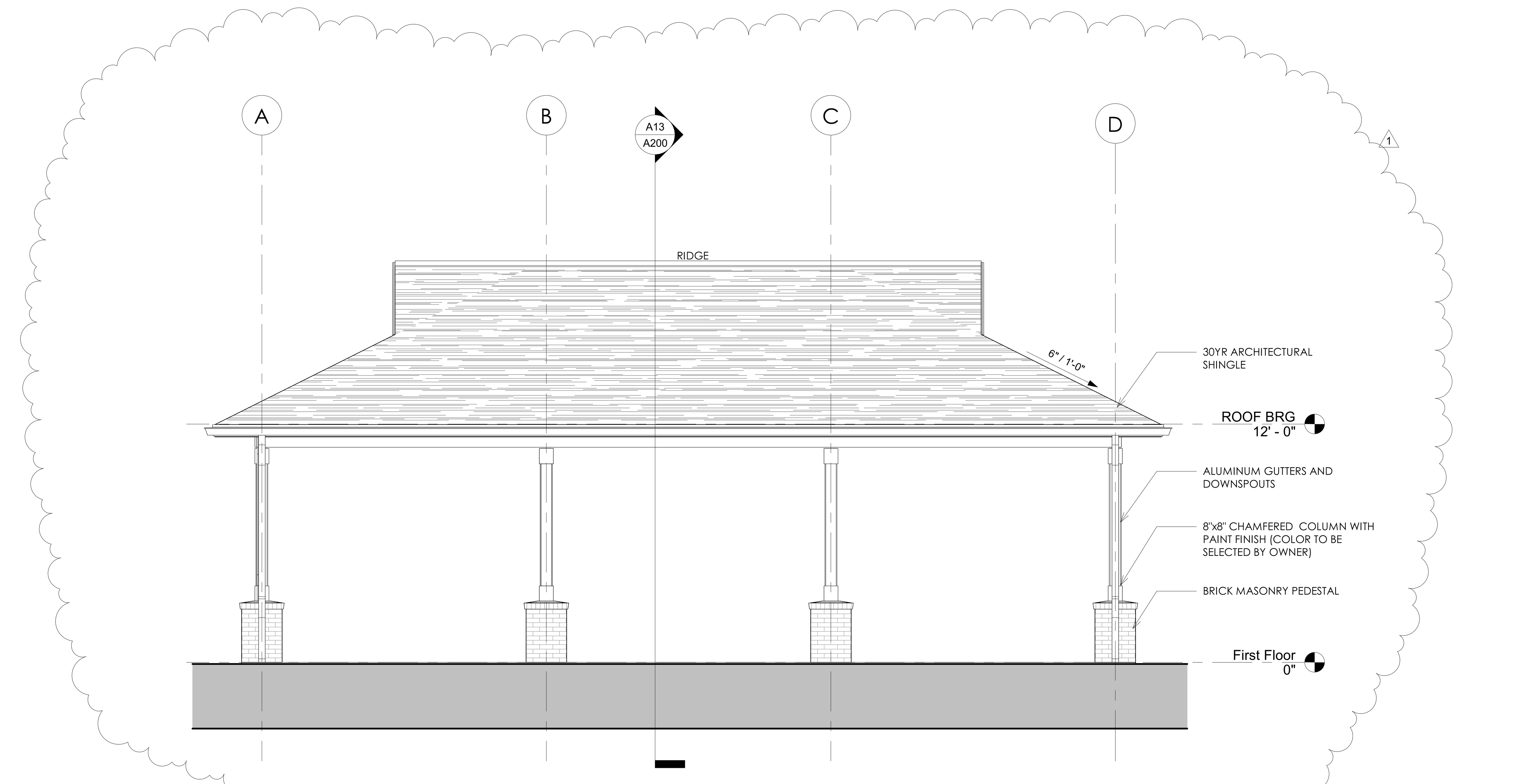
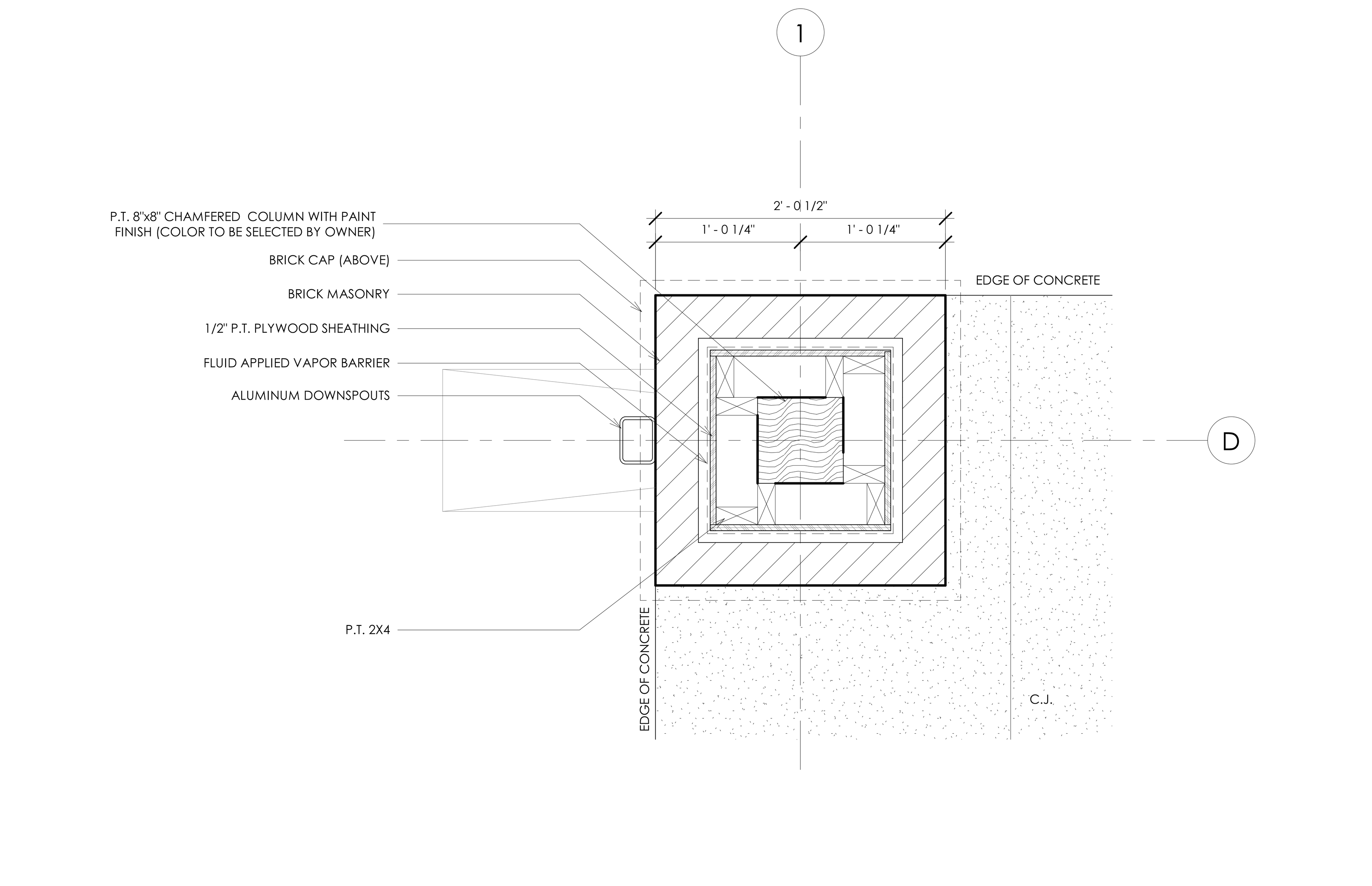
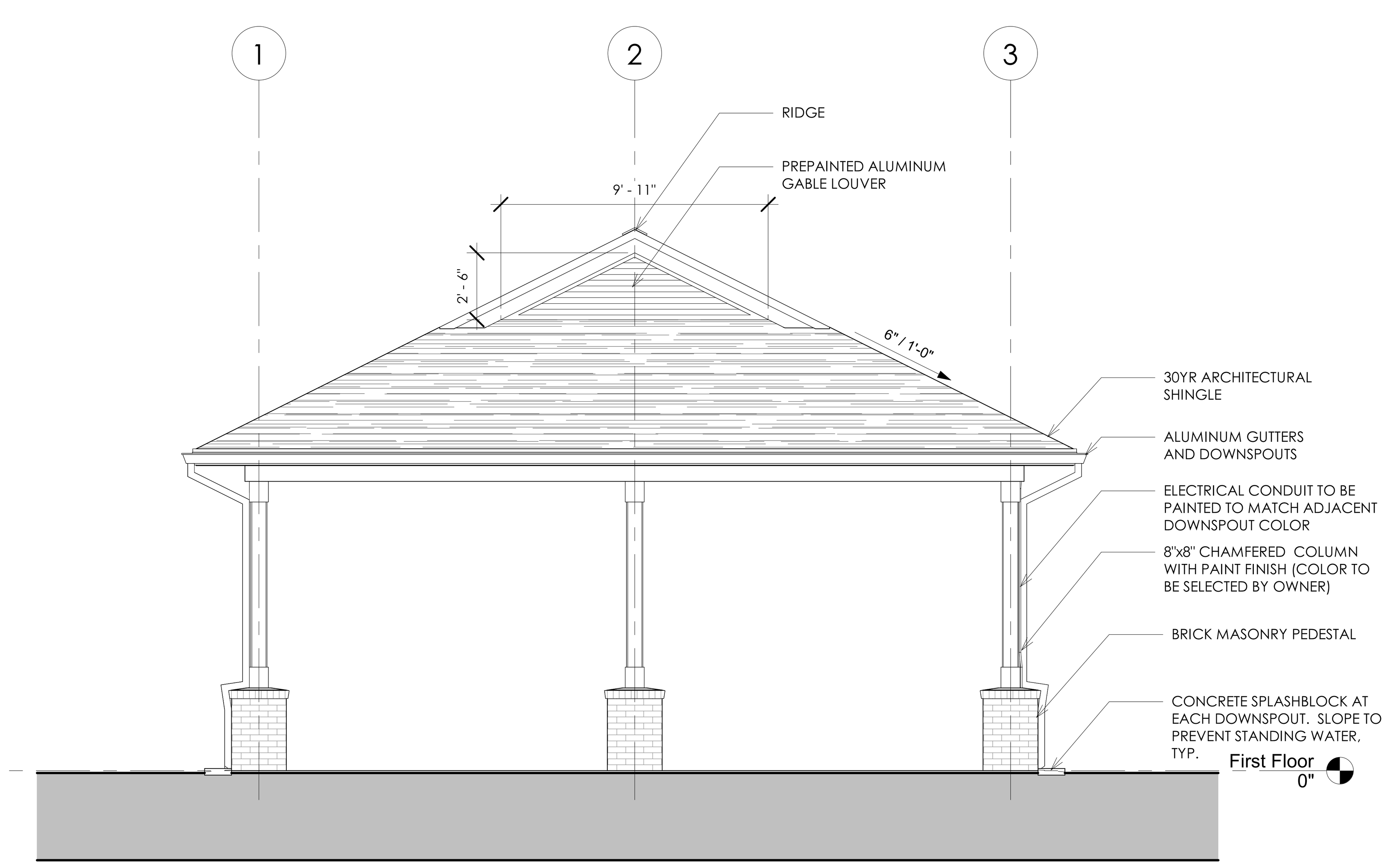


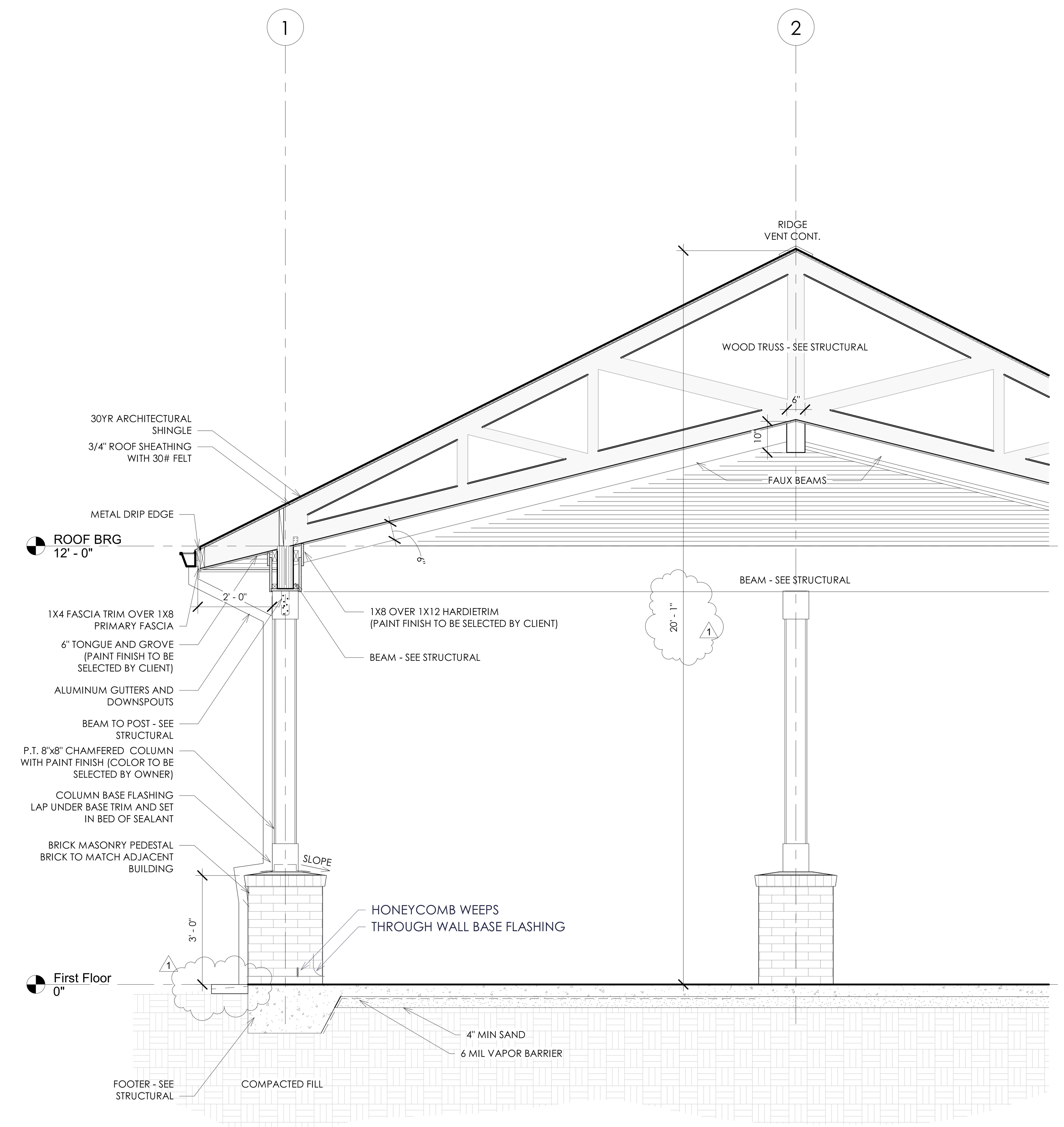
K1 **NORTHELEVATION**
1/4" = 1'-0"



K13 **COLUMN DETAIL**
1 1/2" = 1'-0"



A1 **ELEVATION**
1/4" = 1'-0"



A13 **SECTION**
1/2" = 1'-0"

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE
1	Response to Permitting Comments	7/10/2026

STATE OF GEORGIA REGISTERED ARCHITECT — STEVEN G. STOWERS — 6/17/26

FOR CONSTRUCTION

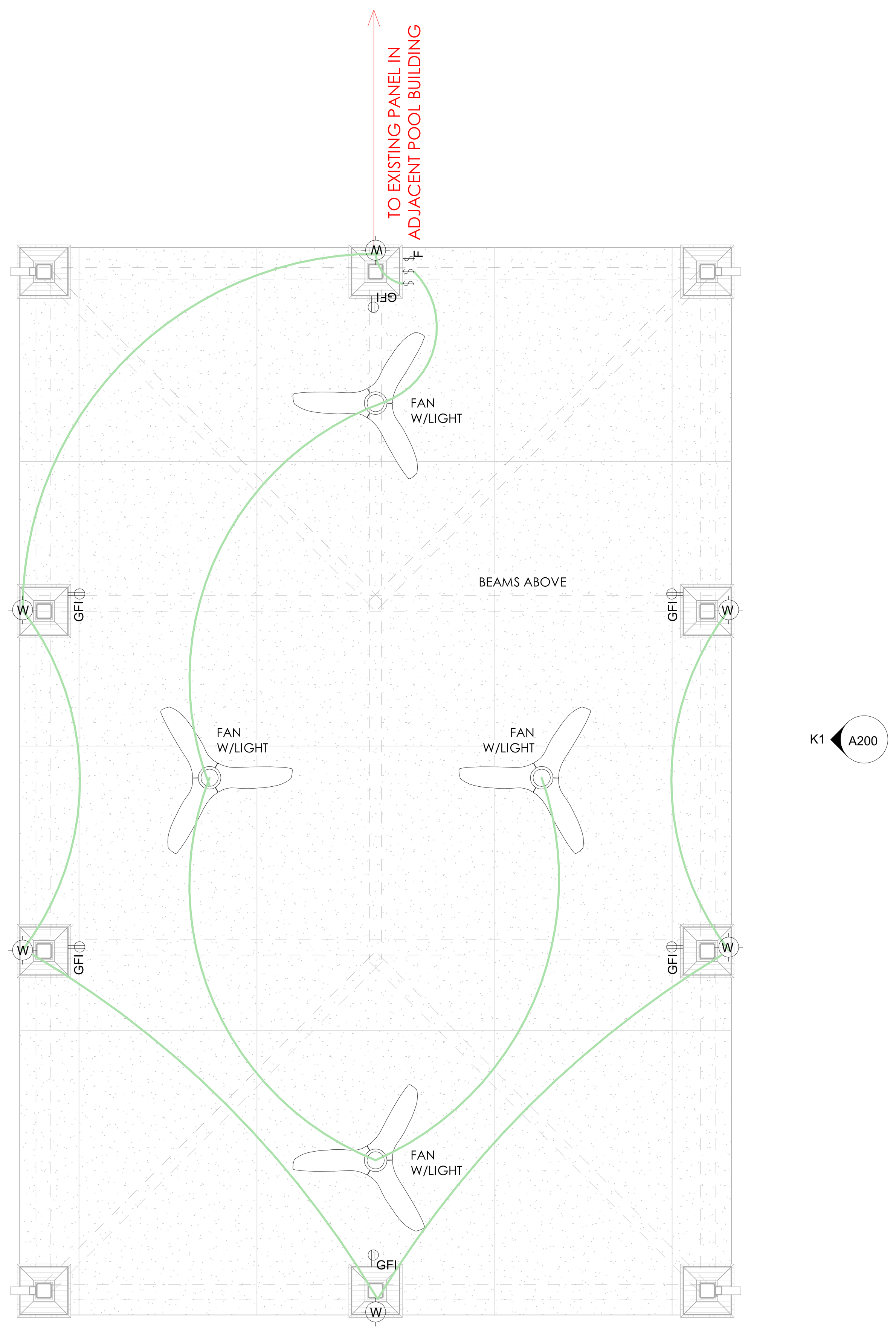
1. **GENERAL CONDITIONS:**
   A. Haul off all construction debris to an approved landfill at regular intervals. Do not allow construction debris to accumulate on site. Dumpsters may be used within fenced in areas, but must be fully covered at all times.
   B. Water can be used from hose bibbs with permission of Savannah Quarters.
   C. Electricity can be used by building's house panel and general service receptacles with permission of Savannah Quarters.
2. **SITEWORK:**
   A. Take special precautions and care during construction to preserve all existing landscaping. Replace any damage landscaping with like size and species.
   B. Provide sod at all disturbed turf areas
3. **TERMITE CONTROL:**
   A. Treat all soil areas under the building against termite infestation.
4. **WOOD FRAMING:**
   A. All exterior wood framing to be pressure treated.
5. **SIDING AND TRIM:**
   A. Field prime all cut joints prior to installation.
   B. Provide stainless steel fasteners throughout.
6. **METAL FLASHING:**
   A. All metal flashing to be factory coated aluminum flashing. Provide self-adhering flashing at top leg of all flashing locations.
7. **EXTERIOR PAINTING AND STAIN:**
   A. Primed exterior siding, T&G, and trim to be painted with two coats of Sherwin Williams Exterior Emerald Rain Refresh, Satin Finish.
   B. Maximum Moisture Content of Substrates:
      a. Fiber Cement Board: 12 Percent
      b. Wood: 15 Percent
   C. All excess paint and stain to be labeled, sealed and to remain on site. Coordinate storage of paint with Owner.

ARCHITECTURE A101
BEAUFORT - SAVANNAH
518 Martin Luther King, Jr. Boulevard | Savannah, Georgia
www.A101.Design | 912.306.0945

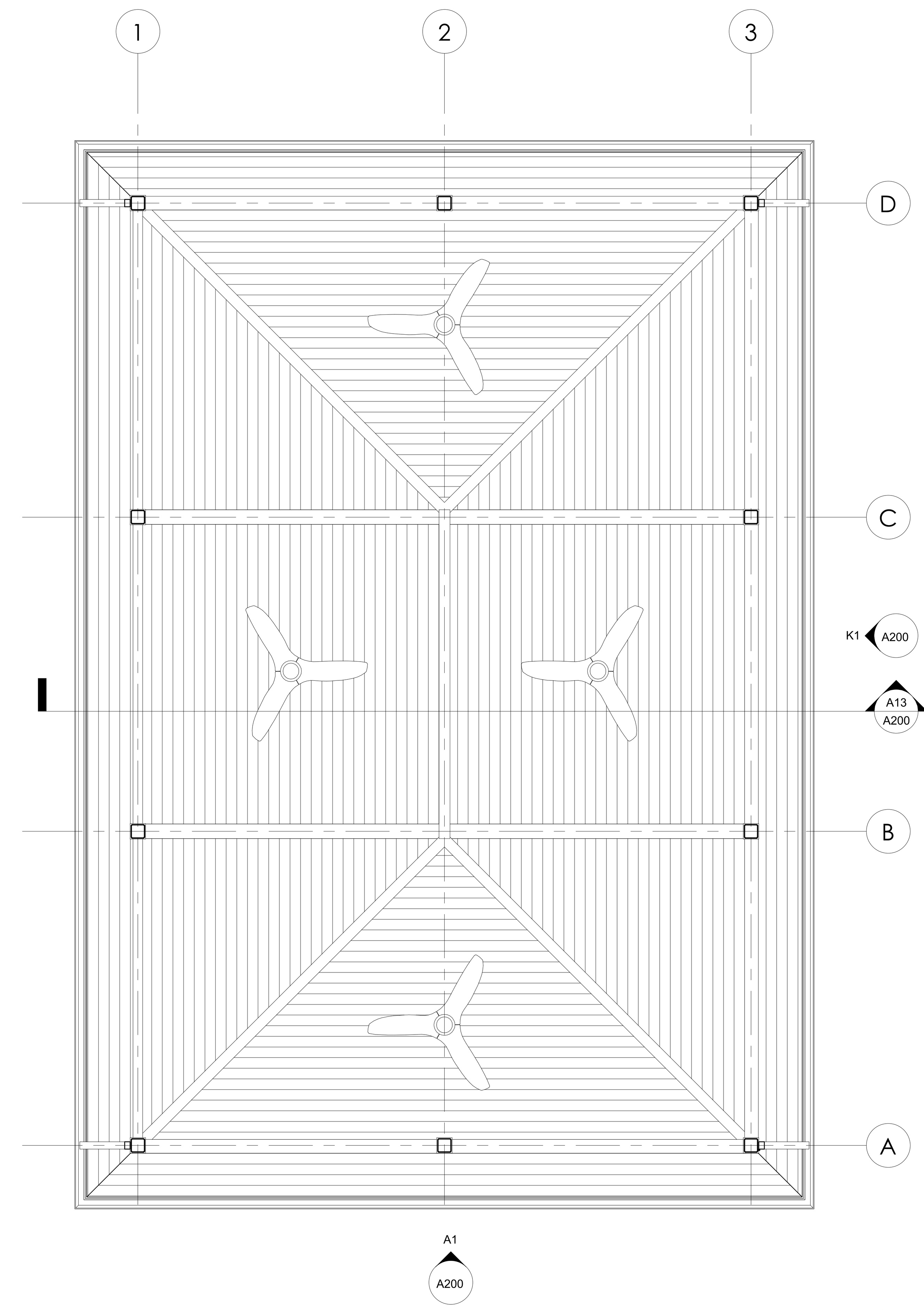
BUILDING ELEVATIONS & SECTION

2026.04
06/17/2026
Drawn By CK
Checked By SGS

A200



A1 **ELECTRICAL FLOOR PLAN**
1/4" = 1'-0"



A12 **FIRST FLOOR - RCP**
1/4" = 1'-0"

SAVANNAH QUARTERS PAVILION
8 Palladian Way, Pooler GA, 31322

REV#	DESCRIPTION	DATE
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STATE OF GEORGIA
STEVEN G. STOWERS
6/17/26
REGISTERED ARCHITECT

FOR CONSTRUCTION

LEGEND

	6" TONGUE AND GROVE (PAINT FINISH TO BE SELECTED BY CLIENT)
W	LIGHT FIXTURE WALL MOUNTED SCONCE - WET LOCATION
S	SINGLE POLE SWITCH LOCATION
GFI	DUPLEX RECEPTACLE - GFI
	CEILING FAN, SEE SPEC SHEET

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ELECTRICAL & RCP PLANS

	2026.04
	06/17/2026
Drawn By	CK
Checked By	SGS

A401

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